
ADDENDUM 01

1. INSTRUCTIONS

This addendum is considered to form part of the contract document issued for this Request for Quotation. Its content precedes previously issued documents in case of conflicting information.

ARCHITECTUAL

SPECIFICATION

- 1.1. **Replace** Specification Section 00 01 10: Table of Contents.
- 1.2. **Add** Appendix C to Specification Section 00 82 30 Request for Quotations Offer
- 1.3. **Add** Specification Section 08 00 00 Schedule of Openings (Hardware)
- 1.4. **Add** Specification Section 09 54 26 Suspended Wood Ceilings.
- 1.5. **Add** Specification Section 10 85 00 Manufactured Specialties
- 1.6. **Add** Specification Section 32 01 91 Chain link fence
- 1.7. **Modify** Specification Section 07 21 13 Board Insulation

1.1 SUMMARY OF WORK

- .1 Slab on Grade:
 - .1 Install **150mm (R-15) Styrofoam SM board insulation 1220mm horizontally** at all perimeter walls.

DRAWINGS

1.8. **A105 Reflected Ceiling Plan**

Under the Reflected Ceiling Plan Legend;
Add the following abbreviation related to specified products;
ACT – Acoustic Tile with suspended ceiling system
GYP – Gypsum suspended ceiling system
LWC – Linear Wood suspended ceiling system

1.9. **A106 Roof Plan & Roof Details**

Include for one additional location to patch in new roofing assembly on the existing roof, 2000mm X 2000mm in area, located where mechanical is removing a roof vent.

ADDENDUM 01

MECHANICAL

Refer to Mechanical Addendum1 dated 07.30.2026 by BPA

ELECTRICAL

Refer to Electrical Addendum1 dated 07.30.2026 by BPA

Refer to Electrical Addendum2 dated 08.06.2026 by BPA

GEOTECHNICAL

Refer to final Geotechnical Investigation dated July 20, 2026 by EXP.

QUESTIONS AND ANSWERS

1. Introduce a cash allowance to cover the costs of filling the area within borehole # 6 with lean mix concrete. Currently there is not enough information to determine the exact volume, should it be in fact required during the construction period.

RESPONSE: Allow for 1 m²

2. Are there specific dates when we can do the work in the existing school? Specifically between gridlines D and F & 1 and 3.

RESPONSE: July 28, 2027 – August 13, 2027

3. Clarify what the separate price for rooms 134, 134A and 134B exactly entail.

RESPONSE: All work

4. Please indicate on the drawing our laydown and parking areas.

RESPONSE: A-001 indicates proposed construction area.

5. Deliveries on electrical distribution equipment have long lead times which will affect the current construction timeframe. Can an extension to the substantial completion date be entertained?

RESPONSE: To be discussed with the successful bidder.

6. Confirm that the only roofing work on the existing building is the one patch noted on drawing A-106.

RESPONSE: There is now a second patch 2000mm X 2000mm roof assembly to match existing.

7. What is the build-up of the existing asphalt areas where the new incoming secondary duct bank is entering the existing building.

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RESPONSE: Allow heavy duty asphalt on granular fill. Area to suit excavation required for duct bank as indicated on electrical documents. Assume that the rock depth along the duct bank as 0.75 m below grade. Allow for excavation of the rock as required for the installation of duct bank.

8. Confirm that no existing trees and/or fence will need to be removed and reinstated where the new secondary duct bank is entering the building.

RESPONSE: Allow 5 meters of fence is to be removed and reinstated.

9. Confirm that note # 1 on drawing A-102 is the only area where the suspended ceiling will need to be removed and reinstalled for all new mechanical and electrical work in the existing building.

RESPONSE: Correct

10. What type of floor covering is required for the existing floor reinstatement between gridlines D and F, along gridline 2 of drawing M-201.

RESPONSE: VCT to match existing colours and patterns. Allow for an area of 25 m² of VCT flooring to be replaced within the existing corridor.

11. What is the existing wall construction for all of the new HVAC penetrations on drawing M-501.

RESPONSE: The existing building exterior wall includes exterior veneer brick and 8" masonry blocks.

12. Confirm that a new suspended ceiling and lights are being installed in room # 134, and that the existing ducts and diffusers are to remain.

RESPONSE: New lights and acoustic tiles. Existing layout of suspended ceiling to remain. Location of diffuses and returns to be modified slightly.

13. Confirm the underside of footing elevation. Drawing C-4 mentions 136.23 which does not coincide with the structural drawings.

RESPONSE: Geotechnical Report and Structural documents shall supersede with respect to footing elevations.

14. Please confirm the construction of the existing play structure that needs to be removed and reinstated. Is it bearing on footings or stringers?

RESPONSE: Assume the existing play structure in on footings. The following Equipment companies are typically used by this School Board; Little Rascals, Bradley Contracting, Parcs.

15. Can the contractor use existing power utility service for construction purposes?

RESPONSE: The contractor and the school board will come to an agreement about how to connect and how to bill the contractor for power usage.

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16. Section 21.7 awards 10 evaluation points to "Contractor with headquarters in Ontario." Would the owner consider revising this criterion to award points to bidders that maintain a permanent office in Ontario, rather than requiring their corporate headquarters to be located in Ontario?

RESPONSE: This is Buy Ontario, we can not modify this close.

17. There are stainless steel clothes hooks, and toilet backrests in the specifications that are not indicated in the drawings. Please specify how many s/s clothes hooks, and toilet backrests are required.

RESPONSE: 1 Clothes hook in each toilet stall, backrest in each barrier free toilet stall (x2)

18. Paper towel dispensers are found in the drawings but not listed in the specifications. Please specify the type of paper towel dispenser and if it is supply and install by contractor.

RESPONSE: Provided by CECCE, installed by Contractor.

19. In the specs, only Rockwool Cavityrock is mentioned for the exterior. Would it be acceptable to use Rockwool Comfortbatt for parapets?

RESPONSE: Comfort Batts are allowed in parapets only.

20. Can you please clarify the spec for LWC, the wood slat ceiling?

RESPONSE: Added

21. Can you please clarify if the ACT in room 134 is new or to be reinstated? It is shown as new on page A105, but reinstated on page A102

RESPONSE: New ACT acoustic ceiling tiles to be installed into existing suspended system. Existing acoustic ceiling tiles to be demolished.

22. Are the 12mm reveal mouldings required in all locations listed in note 6 on page A105? If so, do you have a spec for the product?

RESPONSE: No reveal molding will be required on this project.

END OF ADDENDUM 01

Section 00 01 10

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APPENDIX C

UNIT PRICES

PROJECT NUMBER: 2025JLC117

École élémentaire catholique J.-L. Couroux

Herewith are the Unit Prices

Tender submitted by: _____.

To: Conseil des Écoles Catholiques du Centre - Est

For: École élémentaire catholique J.-L. Couroux

Dated _____ 2026 and which is an integral part of the above noted Tender.

UNIT PRICES

The following unit prices will be applicable for all additional work authorized under this Contract. The Owner reserves the right to negotiate unit prices prior to award of Contract. Each unit price shall include all labour and material involved, all applicable taxes and all overhead and profit pertaining to each item. All unit prices shall exclude the H.S.T.

1. Charges (added cost or less cost) for additional bedrock removal from ground and transported off site.

\$.....per cubic meter

Date _____

Signature _____

Name and Title _____

Section 08 00 00

SCHEDULE OF OPENINGS

2026 ADD. 1

DOOR SCHEDULE											
Door number	Doors				Frames			Hardware group	Fire rating ULC (minutes)	Glazing	Note(s)
	Material	TYPE	Width (mm)	Height (mm)	Material	Width	Depth				
106.1	ST	D1	1015 x2	2135	PS	50	200	04	90	-	
130A.1	ST	D1	965	2135	PS	50	154	03	45	-	
134A.1	WD	D1	965	2135	PS	50	154	02	-	-	WC
134B.1	WD	D1	965	2135	PS	50	154	03	-	-	CL
171.1	WD	D1	965	2135	PS	50	154	01	-	-	
171.2	WD	D1	965	2135	PS	50	154	01	-	-	
171B.1	WD	D1	965	2135	PS	50	154	02	-	-	WC
172.1	WD	D1	965	2135	PS	50	154	01	-	-	
173.1	WD	D1	965	2135	PS	50	154	01	-	-	
174.1	WD	D1	965	2135	PS	50	154	01	-	-	
175.1	WD	D1	965	2135	PS	50	154	01	-	-	
176.1	WD	D1	965	2135	PS	50	154	01	-	-	
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178.1	WD	D1	965	2135	PS	50	154	01	-	-	
179.1	WD	D1	965	2135	PS	50	154	01	-	-	
180.1	WD	D1	965	2135	PS	50	154	01	-	-	
V107.1	AL	D2	1015	2135	AL	-	-	05	-	GL	
V107.2	AL	D2	1015	2135	AL	-	-	06	-	GL	
V107.3	AL	D2	1015 x2	2135	AL	-	-	07	-	GL	

GENERAL NOTES:

- General contractor is responsible for exact measurements of doors and frames
- General contractor must refer to Section 08 71 00 – Door Hardware.
- All wood doors to receive clear poly finish and frames to be painted.
- WD = Wood Door
- PS = Pressed Steel Frame
- ST = Steel Door

Section 08 00 00

SCHEDULE OF OPENINGS

2026 ADD. 1

HARDWARE GROUPS:

Group 01

Quantity	Description
3	Full Mortise Hinge
1	Cylindrical lockset – Classroom with lockdown function
1	Door stop: floor
2	Kickplate 250 mm high

Group 02

Quantity	Description
3	Full Mortise Hinge
1	Cylindrical lockset – Passage latch
1	Door stop: floor
2	Kickplate 250 mm high

Group 03

Quantity	Description
3	Full Mortise Hinge
1	Cylindrical lockset – Storeroom lock
1	Door stop: floor
2	Kickplate 250 mm high

Group 04

Quantity	Description (Per door leaf)
3	Full Mortise Hinge
1	Fire Exit Device 1, Surface mounted vertical rod device c/w top latch
2	Kickplate 250 mm high
1	Electric Controlled Door Holding Magnet

Group 05

Quantity	Description
1	Continuous Hinge
1	Electric Strike
1	Exit device 2
1	Door Pull
1	Key Switch
1	Auto Operator
1	Relay
2	Actuator
2	Escutcheon
1	Surface Closer
1	Mounting Plate

Section 08 00 00

SCHEDULE OF OPENINGS

2026 ADD. 1

1	Overhead door stop (stop only)
1	Threshold
1	Threshold Stop
1	Door Sweep
1	Weather Gasket (head and jambs)

Mode of Operation: Exit devices are dogged open by cylinder during required times, when dogged open entrance / exit is by manually pushing / pulling doors or pushing either side actuator to begin operator cycle. When devices are not dogged open (locked) the exterior actuator is tuned off by key switch mounted on the aluminum frame.

Group 06

Quantity	Description
1	Continuous Hinge
1	Strike
1	Exit device 2
1	Door Pull
1	Key Switch
1	Surface Closer
1	Mounting Plate
1	Overhead door stop (stop only)
1	Threshold
1	Threshold Stop
1	Door Sweep
1	Weather Gasket (head and jambs)

Group 07

Quantity	Description (per door leaf)
1	Continuous Hinge
1	Strike
1	Exit device 2
1	Exit device 3
1	Door Pull
1	Surface Closer
1	Overhead door stop (stop only)
1	Threshold
1	Threshold Stop
1	Door Sweep
1	Weather Gasket (head and jambs)
1	Removable mullion

Section 08 00 00

SCHEDULE OF OPENINGS

2026 ADD. 1

DOOR HARDWARE:

Unless noted otherwise Section 08 71 00

Locks:

Contractor to provide new cinders keyed to match existing master key. Coordinate with CECCE Project Manager for keying.

Locksets:

Schlage ND96PD ANSI F86, Finish: 626 Satin Chromium Plated, Grade 1 Commercial Cylindrical Locksets, with Interchangeable cores, RHO handles.

Fire Exit Device 1, Surface mounted vertical rod device c/w top latch:

3327A-F/3527A-F, 626 Satin Chromium Plated ,360L-BE Lever blank escutcheon, top strike 299F Dull black.

Exit Device 2: CD-35A-NL x388NL OP 626-965, Cylinder to use Existing ASSA cylinder, Keying to existing keying

Exit Device 3: CD-35A-EO-626-965

Electric Controlled Door Holding Magnet:

LCN SEM 7830 Magnet. Surface wall mounted. Aluminum finish. Provide extension kit to suit existing conditions. Coordinate with Electrical Documents for connections to fire alarm panel.

Key Switch: 960N-MA x28

Auto Operator: HA-8 (Push) c/w extended spindle / arm

Relay: CX-33

Actuator: CM60/2

Escutcheon: CM89S

Butt Hinges:

Hager Finish: 626 Heavy duty 127 mm high x 45 mm thick doors. Non removable Pins on Exterior Doors

Continuous Hinges: 780-112 HD Clear

Electric Strike: 6111 12VAC C32D (contractor to confirm voltage)

Removable mullion: 4854-689

Closers:

Heavy Duty LCN (Mount on interior side)

Mounting plate: 4040XP-18G AL

Overhead door stop: 104 C32D

Threshold: CT-44 (thermally broken) Length to suit door opening

Section 08 00 00

SCHEDULE OF OPENINGS

2026 ADD. 1

Threshold Stop: CT-40S

Door Sweep: W-24S-CA

Gasketing: 5100S-96 GRY

Kick Plates:

Standard Metal Finishes: 32D Stainless Steel, satin Style K10C 0.125" thick Countersunk screws Heights as noted.

Floor Stops:

Standard Metal S-Series Finishes: 32D Stainless Steel, satin Dome Floor Stops S103 adjust to suit with Risers as required.

END OF SECTION

1. GENERAL

1.1 RELATED REQUIREMENTS

- .1 Louvres, Intakes and Vents: Air inlets and outlets to be coordinated with ceiling work.

1.2 REFERENCE STANDARDS

- .1 ASTM International
 - .1 ASTM A641: Standard Specification for Zinc Coated (Galvanized) Carbon Steel Wire
 - .2 ASTM C423: Standard Test Method for Sound Absorption and Sound Absorption Coefficients by the Reverberation Room Method
 - .3 ASTM C635: Standard Specifications for Metal Suspension Systems for Acoustical Tile and Lay-In Panel Ceilings
 - .4 ASTM C636: Standard Practice for Installation of Metal Ceiling Suspension Systems for Acoustical Tile and Lay-in Panels
 - .5 ASTM E84: Standard Test Method for Surface Burning Characteristics of Building Materials
 - .6 ASTM E580: Standard Practice for Application of Ceiling Suspension Systems for Acoustical Tile and Lay-in Panels in Areas Requiring Seismic Restraint
 - .7 ASTM E795 – Practice for Mounting Test Specimens During Sound Absorption Tests
- .2 Canadian General Standards Board (CGSB)
 - .1 CAN/ULC-S102 – Method of Test for Surface Burning Characteristics of Building Materials and Assemblies

1.3 QUALITY ASSURANCE

- .1 Single-Source Responsibility: Obtain wood components and products identified on this specification from a single manufacturer, with shop drawing capabilities and resources to provide products of consistent quality in appearance and physical properties without delaying the project.
- .2 Installer Qualification: Must be experienced in the installation of systems similar to those specified herein
- .3 Surface Burning Characteristics: Components tested per ASTM E84/ULC S102
- .4 Flame Spread: 25 or less (Class A)
- .5 Smoke Developed: 450 or less
- .6 Inspection: All work must pass inspection and approval of architect, as well as the local codes and regulations or authorities having jurisdiction.
- .7 Woodworking Standards: Manufacturer must comply with specified provisions of Architectural Woodworking Institute quality standards.
- .8 Design Criteria: Wood components shall be installed true and plumb to within manufacturing tolerance of 1/8" over 8' long.
- .9 Environmental Standards: When required the solid wood, MDF/Particleboard and/or veneer used to produce the wood components shall originate from well managed forests as certified by organizations accredited by the Forestry Stewardship Council.

Section 09 54 26

SUSPENDED WOOD CEILINGS

2026 ADD. 1

Manufacturer shall demonstrate compliance by providing a Chain of Custody (COC) Number.

1.4 ACTION AND INFORMATIONAL SUBMITTALS

- .1 Product Data: Submit manufacturer's data and installation details.
- .2 Shop Drawings: Submit shop drawings showing all areas involved, attachment conditions and perimeter conditions. AutoCAD files containing RCPs, elevations, details and all other relevant information shall be provided at no charge to the manufacturer to facilitate timely and accurate drawings.
- .3 Submittal Samples: Submit representative samples of each material that is to be exposed in the finished work, showing the full range of color and finish variations.

1.5 DELIVERY, STORAGE AND HANDLING

- .1 Material must be stored and installed only in a secured ambient environment (humidity minimum 25%, maximum 55%, temperature range 60°F to 80°F).
- .2 Windows, doors, HVAC and all wet work must be completed before unpacking and installation. Handle carefully to avoid damaging materials.
- .3 Store materials in original, unopened packages in a fully-enclosed space protected against damage from moisture, direct sunlight, surface contamination, pest infestations, and other damage hazards.
- .4 Prior to installation, acclimatize wood components for a minimum of 72 hours to stabilize moisture content and reach room temperature, per AWI standards.
- .5 Handle Acoustic Plank Wood Members carefully to avoid chipping edges or damaging units in any way.

1.6 MAINTENANCE MATERIALS

- .1 Extra Materials: Furnish extra materials described below that match products installed, are packaged with protective covering for storage, and are identified with labels clearly describing contents.
- .2 Wood Components: Furnish full-size components equal to 2.0 percent of amount installed.
- .3 Suspension System Components: Furnish quantity of each component equal to 2.0 percent of amount installed.

1.7 WARRANTIES

- .1 Wood components: Limited one (1) year from date of installation
- .2 Suspension system: Limited ten (10) years from date of installation

2. PRODUCTS

2.1 MANUFACTURER

- .1 CertainTeed Architectural.

Section 09 54 26

SUSPENDED WOOD CEILINGS

2026 ADD. 1

2.2 SUSPENDED WOOD CEILINGS

- .1 Manufacturer: CertainTeed Architectural
- .2 Product: Wood Ceilings
 - .1 Species: MAPLE
 - .2 Finish: CLEAR
 - .3 Substrate: RIFT CUT VENEER
 - .4 Fire Class: A
 - .5 Installation: Direct Mount (2-pc Grid Clip 15/16")
 - .6 Plank Width (Nominal): 6"
 - .7 Reveal Width (Actual): 3/4"
 - .8 Open Reveal Type: Felt
 - .9 CertainTeed Ceilings; heavy duty (HD) suspension system

3. EXECUTION

3.1 EXAMINATION

- .1 Ascertain acceptability of substrates and building conditions under which the ceiling system is to be installed. Do not proceed with the installation until any and all unacceptable conditions have been rectified.
- .2 Do not proceed with installation until unsatisfactory conditions including, but not limited to, wet work such as painting, plastering, or cementing have been completed.
- .3 Ensure HVAC system is operating, supply air and return air is filtered to remove particulates, the building is at the appropriate temperature and humidity, and the space is free from interior construction dust.

3.2 INSTALLATION

- .1 Install the ceiling system in accordance with the following:
 - .1 Manufacturer's printed instructions
 - .2 ASTM C636
 - .3 Ceilings & Interior Systems Construction Association (CISCA) recommendations
 - .4 Applicable local code requirements
 - .5 Approved shop drawings

3.3 CLEANING

- .1 Clean surfaces of wood components and suspension system members per manufacturer's instructions for cleaning. Follow manufacturer's instructions for any touchup of minor finish damage. Remove and replace wood components that cannot be cleaned and/or repaired to permanent damage.

Section 09 54 26

SUSPENDED WOOD CEILINGS

2026 ADD. 1

3.4 INSPECTION

- .1 The Owner shall inspect the installation and product on completion. The manufacturer shall provide repair or replacement of components not conforming to requirements as stated herein and said work will then become bound by the terms of this specification.
- .2 Installation labor for removal and replacement of product improperly installed and not conforming to specified installation instructions as detailed in Part 1 and Part 2 and as shown on plans, shall be the responsibility of the installing Contractor.
- .3 Wood components are a real wood veneer or solid wood product. Every effort is made to maintain the overall appearance; however, natural variations in grain, texture, shade, and/or aging may occur in varying site conditions. For these reasons, the manufacturer cannot guarantee the exact matching of grain, pattern, and/or color.

END OF SECTION

1. GENERAL

1.1 ACTION AND INFORMATIONAL SUBMITTALS

- .1 Provide submittals in accordance with Section 01 33 00 - Submittal Procedures.
- .2 Product Data: Provide manufacturer's printed product literature and data sheets and include product characteristics, performance criteria, physical size, finish and limitations.

1.2 1.2 CLOSEOUT SUBMITTALS

- .1 Provide maintenance data for toilet and bath accessories for incorporation into manual specified in Section 01 78 00 - Closeout Submittals.

1.3 1.3 MAINTENANCE MATERIAL SUBMITTALS

- .1 Provide special tools required for assembly, disassembly or removal for toilet and bath accessories in accordance with requirements specified in Section 01 78 00 - Closeout Submittals. Deliver special tools to Owner.

1.4 1.4 DELIVERY, STORAGE AND HANDLING

- .1 Deliver, store and handle materials in accordance with Section 01 61 00 - Common product requirements and with manufacturer's written instructions.
- .2 Delivery and Acceptance Requirements: deliver materials to site in original factory packaging, labelled with manufacturer's name and address.
- .3 Storage and Handling Requirements:
 - .1 Store materials off ground, in dry location, and in accordance with manufacturer's recommendations in clean, dry, well-ventilated area.
 - .2 Replace defective or damaged materials with new.

2. PRODUCTS

2.1 WINDOW COVERINGS / BLINDS

- .1 Solar shades / 3% openness / without valence.
- .2 Colour: to be selected from full line
- .3 Contact: Stores d'Aujourd'hui
865 Boul. Maloney Est, Gatineau Québec J8P1H3
Tel: (819) 663-0469, ventes@storesdau.ca
- .4 Install on all windows within all classrooms (No corridors, no washroom)

2.2 WINDOW FILM

- .1 3M™ FASARA™ Glass Finishes, SH2MACR-I, matte crystal,
- .2 Install on interior side of window in Female Washroom. Full surface.

Section 10 85 00

MANUFACTURED SPECIALITIES

2026 ADD. 1

2.3 SIGNAGE

- .1 Refer to the attached CECCE signage standards document for fabrication and installation.
SIGNALISATION INTÉRIEURE
- .2 Install Type A on all classrooms (171,172,173,174,175,176,177,178,179,180)
Install Type B, symbol #3 on female washroom 170, symbol #4 on male washroom 135,
symbol #5 on W/C 171B, W/C 134A.
Install Type C on Storage Room (134B, 171A)

3. EXECUTION

3.1 EXAMINATION

- .1 Field measurements are to be taken prior to production and installation.
- .2 Do not commence work until all building conditions are satisfactory.

3.2 INSTALLATION

- .1 Install the ceiling system in accordance with the following:
 - .1 Manufacturer's printed instructions
 - .2 Applicable local code requirements
 - .3 Approved shop drawings

3.3 CLEANING

- .1 Clean surfaces of wood components and suspension system members per manufacturer's instructions for cleaning. Follow manufacturer's instructions for any touch-up of minor finish damage. Remove and replace wood components that cannot be cleaned and/or repaired to permanent damage.

3.4 INSPECTION

- .1 The Owner shall inspect the installation and product on completion. The manufacturer shall provide repair or replacement of components not conforming to requirements as stated herein and said work will then become bound by the terms of this specification.

END OF SECTION

.1 GENERAL

1.1 SUMMARY OF WORK

- .1 Work Included: This section includes materials and installation for chain link fences and gates.

1.2 REFERENCES

- .1 American Society for Testing and Materials International, (ASTM).
 - .1 ASTM A 53/A53M-02, Standard Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-Coated Welded and Seamless.
 - .2 ASTM A 90/A90M-01, Standard Test Method for Weight Mass of Coating on Iron and Steel Articles with Zinc or Zinc-Alloy Coatings.
 - .3 A653/A653M-03, Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process.
 - .4 ASTM C 618-03, Standard Specification for Coal Fly Ash and Raw or Calcined Natural Pozzolan for Use as a Mineral Admixture in Concrete.
 - .5 ASTM F 1664-01, Standard Specification for Poly(Vinyl Chloride) (PVC)-Coated Steel Tension Wire Used with Chain-Link Fence.
- .2 Canadian General Standards Board (CGSB).
 - .1 CAN/CGSB-138.1-96, Fabric for Chain Link Fence.
 - .2 CAN/CGSB-138.2-96, Steel Framework for Chain Link Fence.
 - .3 CAN/CGSB-138.3-96, Installation of Chain Link Fence.
 - .4 CAN/CGSB-138.4-96, Gates for Chain Link Fence.
 - .5 CAN/CGSB-1.181-99, Ready-Mixed Organic Zinc-Rich Coating.
- .3 Canadian Standards Association (CSA International).
 - .1 CAN/CSA-A23.1/A23.2-00(August 2001), Concrete Materials and Methods of Concrete Construction/Methods of Test for Concrete.
 - .2 CAN/CSA-G164-M92(R2003), Hot Dip Galvanizing of Irregularly Shaped Articles.
 - .3 CAN/CSA-A3000-98(R2002), Cementitious Materials Compendium. Includes:
 - .1 CAN/CSA-A23.5-98, Supplementary Cementing Materials

1.3 SUBMITTALS

- .1 Submittals in accordance with Section 01 33 00 - Submittal Procedures .
- .2 Submit WHMIS MSDS - Material Safety Data Sheets.
- .4 Shop Drawings to indicate: all hardware components.
- .5 Paint specs – provide paint procedures and materials data sheets for review.

Section 32 01 91

CHAIN LINK FENCE

2026 ADD. 0

.2 PRODUCTS

2.1 MATERIALS

- .1 Concrete mixes and materials:
 - .1 Nominal coarse aggregate size: 20mm maximum size, -5mm minimum size.
 - .2 Compressive strength: 27 MPa minimum at 28 days.
 - .3 Water/cement ratio to CAN3-A23.1-M77 Table 7 for Class A exposure and 60 mm slump at time and point of deposit. Air entrainment to CAN3-A23.1-M77 Table 8.
- .2 2400mm ht sections: Posts, braces and rails: to CAN/CGSB-138.2, galvanized steel pipe, schedule 40. Dimensions as indicated.
 - .1 Top, Mid and Bottom Rails: 42.8mm O.D., 3.56mm wall thickness.
 - .2 Line Posts: 88.9mm O.D., 5.49mm wall thickness.
 - .3 Corner / End Posts: 114mm O.D., 5.49mm wall thickness.
- .3 Tie wire fasteners: to CSA G162.1M. Single strand, galvanized steel wire conforming to requirements of fence fabric.
- .4 Tension bar: to ASTM A 653/A653M, 5 x 20 mm minimum galvanized steel.
- .5 Tension bar bands: 3 x 20 mm minimum galvanized steel or 5 x 20 mm minimum aluminium,
- .6 Fittings and hardware: to CAN/CGSB-138.2, cast aluminum alloy or galvanized steel
 - .1 Tension bar bands: 3 x 20 mm minimum galvanized steel or 5 x 20 mm minimum aluminum.
 - .2 Post caps to provide waterproof fit, to fasten securely over posts and to carry top rail.
 - .3 Turnbuckles to be drop forged.
- .10 Rubber caps: Provide suitable caps to bolts on the inside of all bolts used in the kindergarten play area. Provide sample for approval.
- .11 Organic zinc rich coating: to CAN/CGSB-1.181.

2.2 FINISHES

- .1 Galvanizing:
 - .1 For chain link fabric: to CAN/CGSB-138.1, 1.2 oz. (34 grams).
 - .2 For pipe: 550 g/m2 minimum to ASTM A90.
 - .3 For other fittings: to CAN/CSA-G164.

.3 EXECUTION

3.1 GRADING

- .1 Remove debris and correct ground undulations along fence line to obtain smooth uniform gradient between posts.
 - .1 Bottom of fence to be flush with ground surface where fence is installed in asphalt.
 - .2 Provide clearance between bottom of fence and ground surface of 30 mm to 60 mm.

Section 32 01 91

CHAIN LINK FENCE

2026 ADD. 0

3.2 ERECTION OF FENCE

- .1 Erect fence along lines as indicated or as directed by Consultant and to CAN/CGSB-138.3.
- .2 Excavate post holes to the following depths: 1100mm depth (of concrete) x 300mm diameter for line posts, and 1250mm depth (of concrete) x 350mm diameter for end, corner, and gate posts.
- .3 Space line posts 3 m apart, measured parallel to ground surface.
- .4 Space straining posts at equal intervals not to exceed 150 m if distance between end or corner posts on straight continuous lengths of fence over reasonably smooth grade, is greater than 150m.
- .5 Install additional straining posts at sharp changes in grade and where directed by Owner or Consultant.
- .6 Install corner post where change in alignment exceeds 10 degrees.
- .7 Install end posts at end of fence and at buildings.
 - .1 Install gate posts on both sides of gate openings.
- .8 Cut both sides of bottom section of sonotubes to allow bottom section to expand. Place sonotubes to bottom of post hole and 150 mm below grade. Place concrete in post holes then embed posts into concrete to the following depths: 840 mm depth for line posts, 910mm for end, corner, and gate posts. Finish concrete 150 mm below ground level and slope to drain away from posts. Brace to hold posts in plumb position and true to alignment and elevation until concrete has set. Fill remaining 150 mm with compacted screened topsoil and sod as required or finish concrete to top of asphalt surface & trowel smooth.
- .9 Do not install fence fabric until concrete has cured minimum of 5 days.
- .10 Install brace between end and gate posts and nearest line post, placed diagonally across panel.
 - .1 Install braces on both sides of corner and straining posts in similar manner.
- .11 Install overhang tops and caps.
- .12 Install top rail between posts and fasten securely to posts and secure waterproof caps and overhang tops.
- .13 Install bottom rail between posts and fasten securely to posts.
- .14 Lay out fence fabric on inside of enclosed area. Stretch tightly to tension recommended by manufacturer and fasten to end, corner, gate and straining posts with tension bar secured to post with tension bar bands spaced at 300 mm intervals.
 - .1 Knuckled selvedge at bottom.
 - .2 Twisted selvedge at top.
- .15 Secure fabric to top rails, line posts and bottom rails with tie wires at 450 mm intervals.
 - .1 Give tie wires minimum two twists.

Section 32 01 91

CHAIN LINK FENCE

2026 ADD. 0

- .16 Confirm by pull tests that a 12 kg perpendicular pull at mid-point of each mesh panel will displace mesh max. 50 mm from vertical plans.
- .17 Face collar nuts to compound interior and torque tight.
 - .1 For fences at kindergarten and daycare play areas:
 - .1 Grind bolts to expose a three threads and sand smooth with no burs or pointe ends.
 - .2 Add rubber caps or ends so that no exposed bolt can pose a safety hazard for children playing. Have rubber caps approved.

3.3 TOUCH UP

- .1 Clean damaged surfaces with wire brush removing loose and cracked coatings. Apply two coats of organic zinc-rich paint to damaged areas.
- .2 Pre-treat damaged surfaces according to manufacturers' instructions for zinc-rich paint.

3.4 3.5 CLEANING

- .1 Clean and trim areas disturbed by operations.
- .2 Dispose of surplus material and replace damaged turf with sod or as directed by Owner or Consultant.

END OF SECTION

No. MEC-001

Client: Conseil des écoles catholiques du Centre-Est
Project: École Élémentaire Catholique J.-L. Couroux
Project No.: 25-1159
Description: Revisions to drawings
Division(s): 22 and 23
By: Ayman Kassan, P.Eng.



Date: 2026-07-30

This addendum forms an integral part of the original plans, specifications, and contractual documents. Bidders shall make sure that the cost of this addendum is included in the bid amount.

1. Documents

1.1 Included Documents

- Mechanical Plans:
 - M-101, revision 1
 - M-203, revision 1
 - M-206, revision 1

2. Description of Work

2.1 Drawing M-101

- Changes made to the faucet schedules in the Plumbing Schedule.
- Hot water Tank BMS requirements have been removed from the Hot Water Tank Schedule.
- RTU-08 cooling capacity change.

2.2 Drawing M-203

- Modifications to Drawing Note 2 with the addition of the vent piping serving the hot water tank within the demolition scope

2.3 Drawing M-206

- Addition of General Note A regarding seismic supports for new natural gas piping.

For details, see attached documents.

PLUMBING SCHEDULE													
REF. NO	TYPE	MOUNTING	MATERIAL	DESIGNED BASED ON (APPROVED EQUIVALENTS ACCEPTED)	CHARACTERISTICS	CARRIER	CONNECTIONS			NOTES			
					MIN. W.C.	MAX. W.C.	IN.	DCW	DHW	SANITARY			
L-1	LAVATORY	WALL	VITREOUS CHINA	AMERICAN STANDARD, 0955001EC.020, 005902EC.020	MOEN, 8894SP	-	-	WATTS, CA-411-WC	NPS 1/2	NPS 1/2	NPS 1 1/2	C/W THERMOSTATIC MIXING VALVE, FAUCET POWER KIT, STRAIGHT FIXTURE DRAIN, P-TRAP AND ACCESSORIES	
S-1	SINK	COUNTER MOUNT	STAINLESS STEEL	FRANKE, LBS4608P-1	ZURN, Z867C0-XL	-	-	NPS 3/4	NPS 3/4	NPS 1 1/2	C/W THERMOSTATIC MIXING VALVE, FAUCET POWER KIT, P-TRAP AND ACCESSORIES		
S-2	SINK	COUNTER MOUNT	TERREON	BRADLEY, TLX-2, EMPIRE GRAY FINISH	BRADLEY, SSS-3300, POLISHED CHROME	BRADLEY, S17-382	NPS 3/4	NPS 3/4	NPS 1 1/2	C/W THERMOSTATIC MIXING VALVE, AC ADAPTER A14-029, P-TRAP AND ACCESSORIES			
S-3	SINK	COUNTER MOUNT	STAINLESS STEEL	FRANKE, LBS6110P-1	KOHLER K-22973	-	NPS 3/4	NPS 3/4	NPS 1 1/2	C/W THERMOSTATIC MIXING VALVE, FAUCET WITH SPRAY HEAD, P-TRAP AND ACCESSORIES			
WC-1	WATER CLOSET	FLOOR	VITREOUS CHINA	AMERICAN STANDARD, 5401001.020	DELTA, 81T201HWA-MMO	-	NPS 1	-	NPS 4	C/W TOILET SEAT BEMIS (1955CT 000), ELECTRONIC FLUSH, TRANSFORMER AND ACCESSORIES			
U-1	URINAL	WALL	VITREOUS CHINA	AMERICAN STANDARD, 6590001.020	DELTA, 81T231HWA-05-MMO	WATTS, CA-311	NPS 3/4	-	NPS 2	C/W ELECTRONIC FLUSH, TRANSFORMER, CLEANOUT AND ACCESSORIES			
DF-1	DRINKING FOUNTAIN	WALL	STAINLESS STEEL	ELKAY, EZSDWSVRSK	-	ELKAY, MLP100	NPS 1/2	-	NPS 1 1/2	C/W FILTER SYSTEM EWF3000, CANE APRON LKAPREZL AND ACCESSORIES			

HOT WATER TANK SCHEDULE														
IDENTIFICATION	MODEL	MANUFACTURER	CAPACITY (INPUT) MBH	THERMAL EFFICIENCY %	TANK CAPACITY USG	TOTAL RECOVERY RATE @100°F GPH	PRESSURE MIN. W.C.	PRESSURE MAX. W.C.	VENT in.	COMBUSTION AIR in.	SOURCE	HEIGHT in.	WIDTH in.	WEIGHT lbs.
HWT-01&02	BTH-150(A)	A.O. SMITH	150	97	100	173	4	14	3	3	NATURAL GAS	76 1/2	27 3/4	523

PUMP SCHEDULE							
IDENTIFICATION	ROOM ID	DESCRIPTION	MODEL	FLOW GPM	HEAD LOSS FT	MOTOR HP	ELECTRICAL V / Ph / Hz
RP-01	158	HOT WATER RECIRCULATION PUMP	GRUNDFOS UPS 15-55 SFC	2.4	5	87 W	115/1/60
SP-01	180	SUBMERSIBLE PUMP	SANISWIFT PRO #022	18	14	1/3	115/1/60

FAN SCHEDULE									
IDENTIFICATION	DESCRIPTION	LOCATION	MANUFACTURER	MODEL	AIR FLOW		MOTOR HP	FAN SPEED RPM	ELECTRICAL V/PH/Hz
					CFM	IN W.C.			
EF-03	ROOF MOUNTED	ROOF	GREENHECK	G-080-VG	280	0.20	1/10	1,725	115/1/60
EF-08	ROOF MOUNTED	ROOF	GREENHECK	G-080-VG	280	0.20	1/10	1,725	115/1/60
TF-01	CEILING MOUNTED	STORAGE ROOM	GREENHECK	CSP-A110	100	0.15	1/50	937	115/1/60

VAV SCHEDULE									
ID	ROOM ID	ROOM NAME	AIR FLOW		UNIT SIZE	INLET SIZE DIA (mm)	HEIGHT (H) mm	WIDTH (W) mm	LENGTH (L) mm
			MIN (CFM)	MAX (CFM)					
VAV-01	178	CLASS RM	260	653	8	200	250	300	503
VAV-02	179	CLASS RM	250	625	8	200	250	300	503
VAV-03	180	ART	298	745	8	200	250	300	503
VAV-04	177	CLASS RM	234	585	7	175	250	300	503
VAV-05	176	CLASS RM	226	565	7	175	250	300	503
VAV-06	175	CLASS RM	226	565	7	175	250	300	503
VAV-07	107	CORRIDOR	430	1,660	12	300	375	400	503
VAV-08	174	STAFF	64	205	4	100	200	300	553
VAV-09	173	SPEC ED	150	375	6	150	200	300	553
VAV-10	172	SPEC ED	152	380	6	150	200	300	553
VAV-11	171	KINDERGARTEN	297	685	8	200	250	300	503
CAV-01	170	FEMALE W/C	165	165	4	100	200	300	553

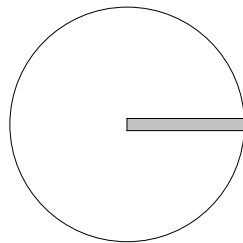
RTU SCHEDULE (DESIGN BASED ON RNA MODELS BY AAO)																			
IDENTIFICATION	DESCRIPTION	LOCATION	SUPPLY FAN		EXHAUST FAN		OUTSIDE AIR	COOLING SECTION				HEATING SECTION		WEIGHT	HEATING TYPE	ELECTRICAL		NOTES	
			AIR FLOW	EXTERNAL STATIC PRESSURE	AIR FLOW	EXTERNAL STATIC PRESSURE		TOTAL CAPACITY	HEAT WHEEL CAPACITY	MIXED AIR TEMP.	OUTPUT	INPUT							
			CFM	in. wg.	CFM	in. wg.	CFM	MBH	MBH	DB (°F)	WB (°F)	CFM	MBH			lbs.	PRIMARY		V/PH/Hz
RTU-08	ROOF TOP UNIT	ROOF	3635	0.85	3635	0.55	1852	119.9	82.3	42.9	76.9	63.7	158	195	2446	NATURAL GAS	208/3/60	61	C/W ENERGY RECOVERY WHEEL, VFD's, DISCONNECT SWITCH AND 24IN SEISMIC ROOFCURB.
RTU-09	ROOF TOP UNIT	ROOF	3295	0.85	3295	0.55	2079	100.2	80	47	77.6	64.3	158	195	2427	NATURAL GAS	208/3/60	58	C/W ENERGY RECOVERY WHEEL, VFD's, DISCONNECT SWITCH AND 24IN SEISMIC ROOFCURB.



SEALS



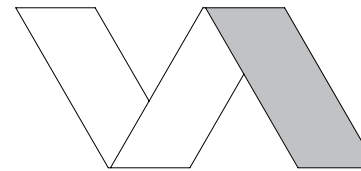
PROJECT NORTH



CLIENT



ARCHITECT



VESR ARCHITECTURE INC.
tel. 613-757-0347 email: info@vesrarch.com
340 Gladstone Ave., Suite 301, Ottawa ON K2P 0Y8

1	2026-07-30	ISSUED FOR ADDENDUM MEC-001	A.K.
0	2026-05-22	ISSUED FOR PERMIT AND TENDER	A.K.
No.	DATE	REVISION	BY

PROJECT

ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUX
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

DRAWING TITLE

MECHANICAL
GENERAL
EQUIPMENTS SCHEDULES

DRAWING A.HANNA	DESIGN A.HANNA	VERIFICATION A.KASSAN
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BPA PROJECT No

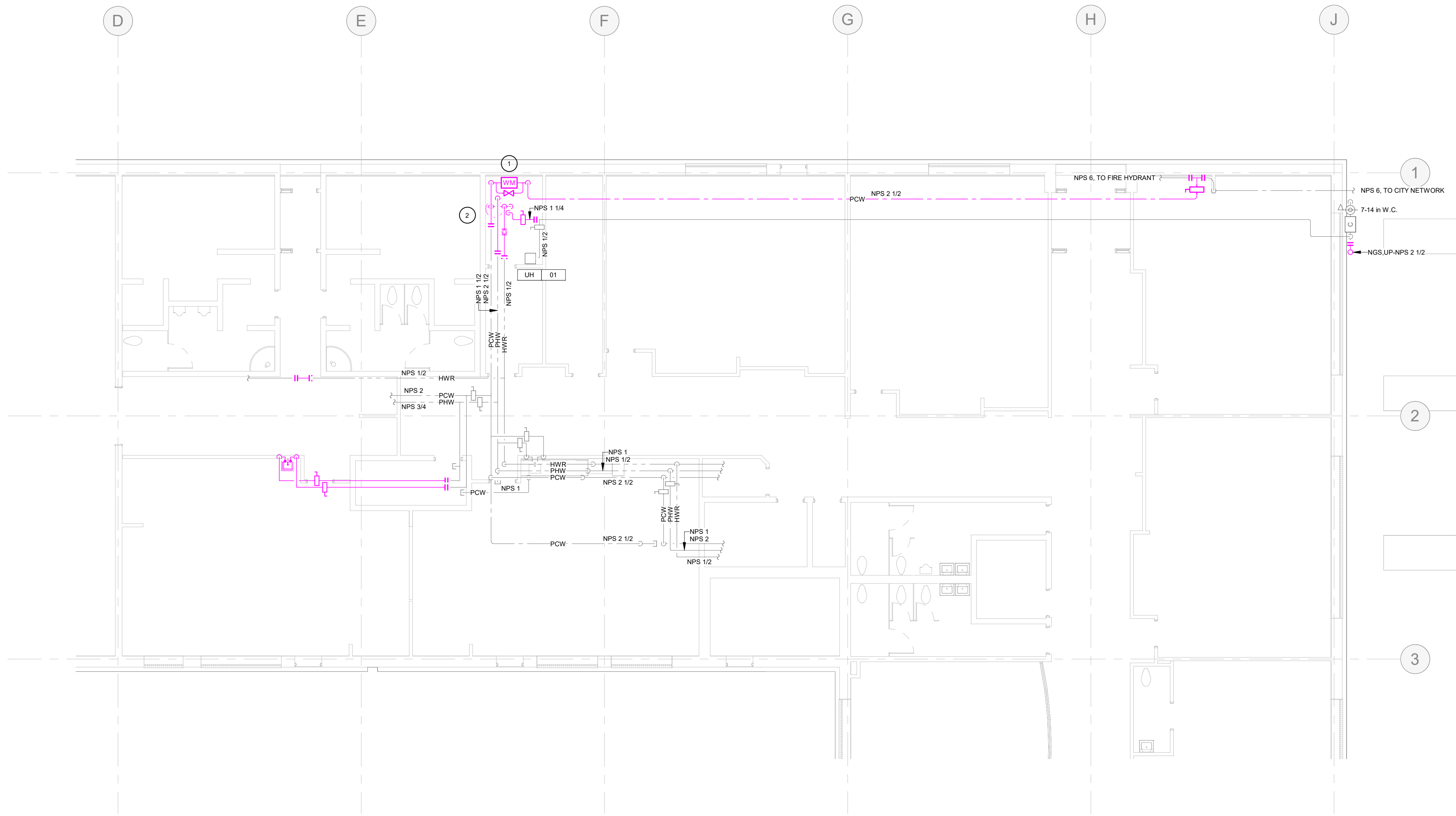
DRAWING No

25-1159

SCALE

NTS

M-101

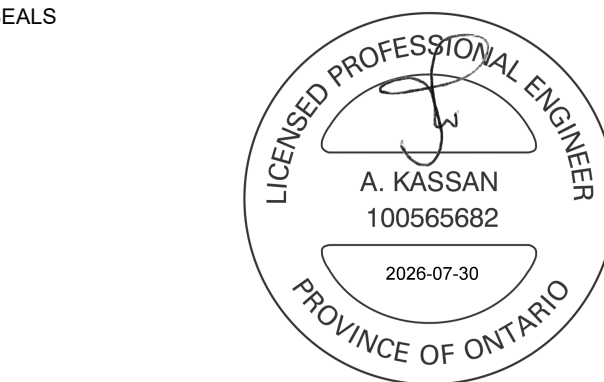


GENERAL NOTES

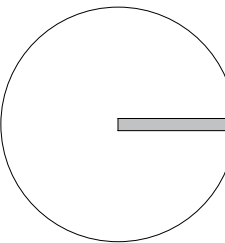
A. THIS DRAWING SHOWS THE GENERAL ARRANGEMENT OF PIPING AND EQUIPMENT. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING DISTANCES AND OBSTRUCTIONS ON SITE. CONTRACTOR MUST CONFIRM THE DIMENSION OF THE PIPING BEFORE ORDERING ANY MATERIAL.

B. SUPPLY AND INSTALL TRAP SEAL PRIMER SYSTEMS FOR ALL NEW FLOOR DRAINS.

- DRAWING NOTES
- REMOVE THE EXISTING WATER METER AND RELOCATE IT TO THE NEW LOCATION AS SHOWN ON SHEET MEC-001.
 - DEMOLISH EXISTING HOT WATER STORAGE TANK CW VENT PIPING UP TO THE ROOF, THE HOT WATER RECIRCULATION PUMP AND ACCESSORIES.



PROJECT NORTH



CLIENT



ARCHITECT



No.	DATE	REVISION	BY
1	2026-07-30	ISSUED FOR ADDENDUM MEC-001	A.K.
0	2026-05-22	ISSUED FOR PERMIT AND TENDER	A.K.

PROJECT

ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUX
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

DRAWING TITLE

MECHANICAL
POTABLE WATER & NATURAL
GAS
GROUND FLOOR
DEMOLITION

DRAWING	DESIGN	VERIFICATION
A.HANNA	A.HANNA	A.KASSAN

BPA PROJECT No

25-1159

SCALE

1 : 100

M-203

COLOUR LEGEND
DEMOLITION AND EXISTING
PROJECT DRAWINGS MUST BE PRINTED IN COLOUR FOR CLARITY.
EXISTING
DEMOLITION

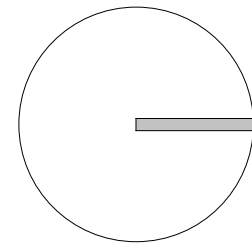
GENERAL NOTES
A. ALL NEW GAS PIPING TO BE SUPPORTED AND SEISMICALLY ANCHORED TO THE ROOF STRUCTURE AS PER CBC REQUIREMENTS.



SEALS



PROJECT NORTH



CLIENT



ARCHITECT



1	2026-07-30	ISSUED FOR ADDENDUM MEC-001	A.K.
0	2026-05-22	ISSUED FOR PERMIT AND TENDER	A.K.
No.	DATE	REVISION	BY

PROJECT
ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUX
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

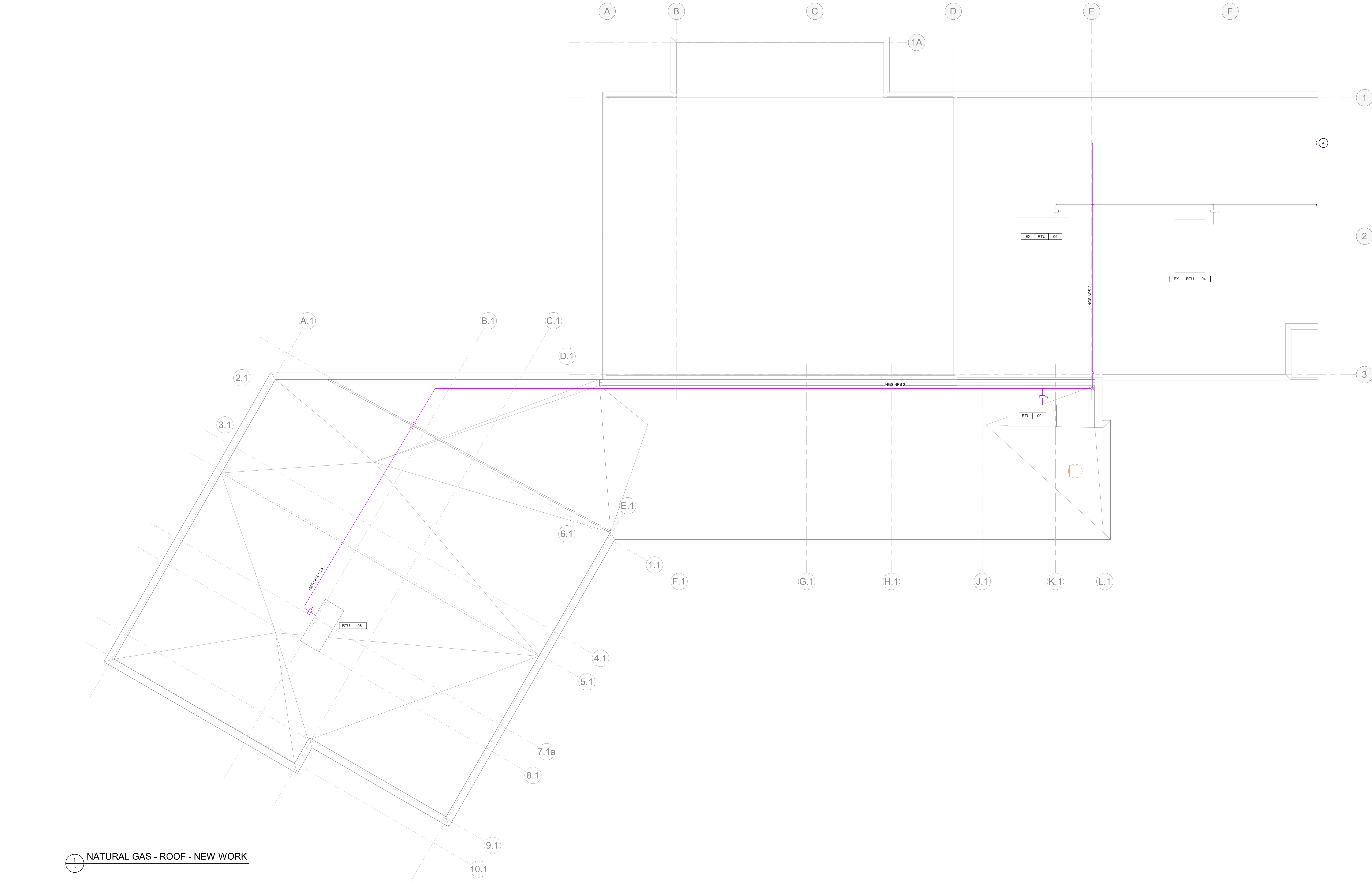
DRAWING TITLE
MECHANICAL
NATURAL GAS
ROOF
NEW WORK

DRAWING A.HANNA	DESIGN A.HANNA	VERIFICATION A.KASSAN
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BPA PROJECT No
25-1159
SCALE
1 : 100

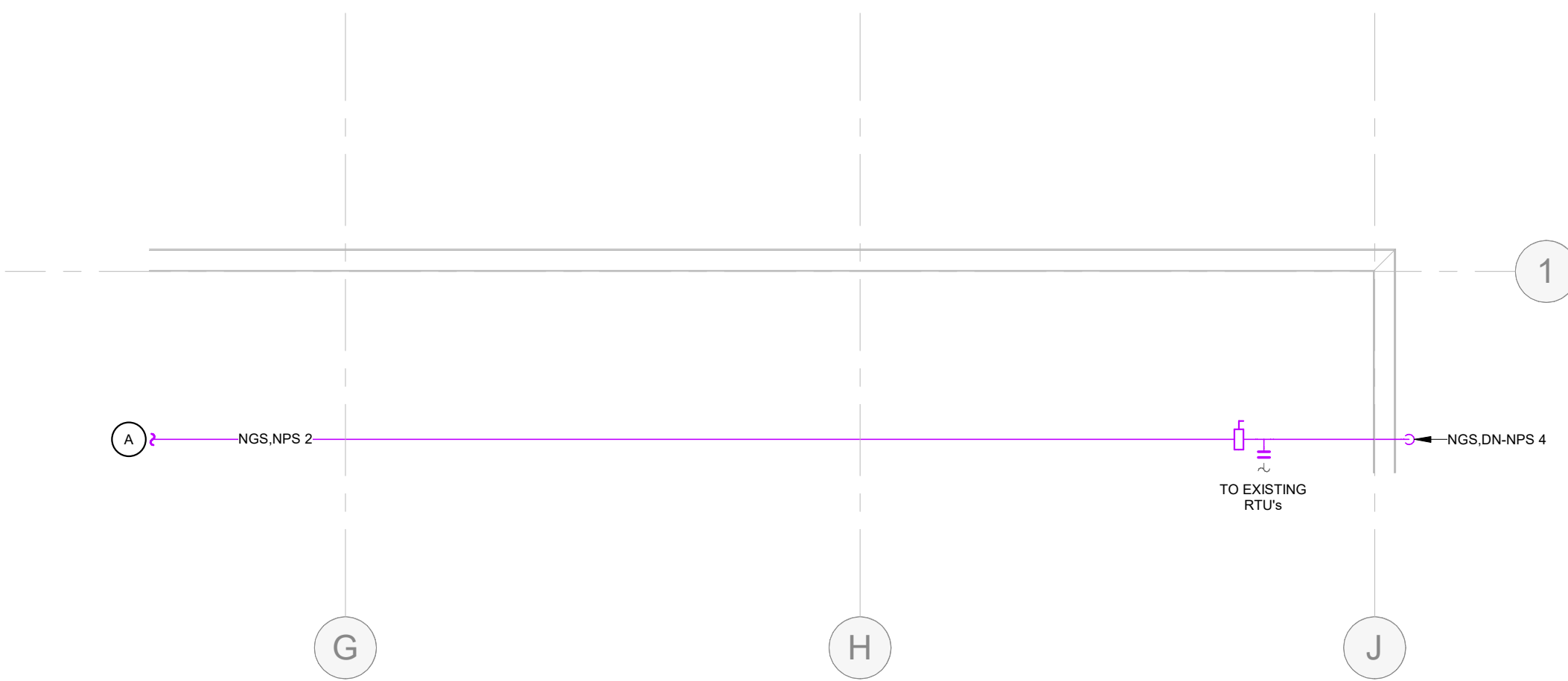
M-206

COLOURS LEGEND
POTABLE WATER
PROJECT DRAWINGS MUST BE PRINTED IN COLOUR FOR CLARITY.
NATURAL GAS
EXISTING



1 NATURAL GAS - ROOF - NEW WORK

2 NATURAL GAS - ROOF - NEW WORK



No. ELE-001

Client:	Conseil des écoles catholiques du Centre-Est	
Project:	École Élémentaire Catholique J.-L. Couroux	
Project No.:	25-1159	
Description:	Revisions to drawings	
Division(s):	26	
By:	Jaroslav Cichocki, C. Tech April Waddell, P. Eng	Date: 2026-07-30

This addendum forms an integral part of the original plans, specifications, and contractual documents. Bidders shall make sure that the cost of this addendum is included in the bid amount.

1. Documents

1.1 Included Documents

- Electrical Drawings:
 - E-000, revision 1
 - E-101, revision 1
 - E-102, revision 1
 - E-201, revision 1
 - E-301, revision 1

2. Description of Work

2.1 Drawing E-000

- Existing underground services shown for information purposes. Refer to general notes for instructions. The new secondary duct bank shall be coordinated around the existing services.

2.2 Drawing E-101

- Revise lighting control details as indicated.

2.3 Drawing E-102

- Revise panel schedules as indicated.

2.4 Drawing E-201

- Revise lighting controls layout as indicated.

2.5 Drawing E-301

- Remove hand dryers in washrooms 134A and 171B.

3. Clarifications

3.1 Questions:

- **Q:** *Confirm that no existing underground services are in the way of the new secondary ductbank.*
A: The new electrical duct bank installation requires coordination onsite with existing underground utilities, such as the watermain, sanitary, gas and bell service. All existing co-located services are to be protected and maintained.

For details, see attached documents.

MISCELLANEOUS

ELECTRICAL DISTRIBUTION

LIGHTING CONTROLS

EMERGENCY LIGHTING & SIGNAGE

FIRE ALARM SYSTEM

IDENTIFICATION

GENERAL

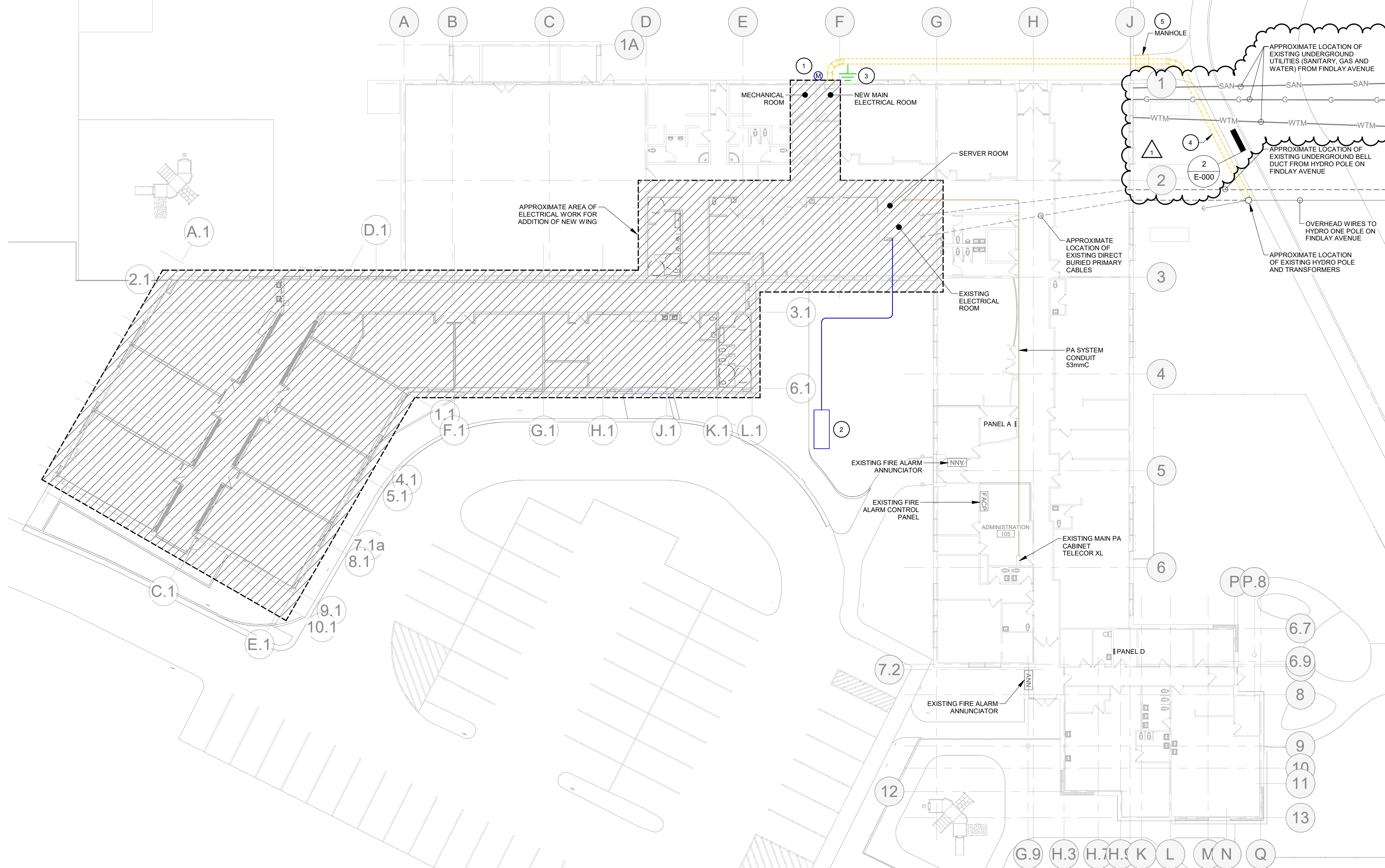
EQUIPMENT IDENTIFICATION

OUTLETS

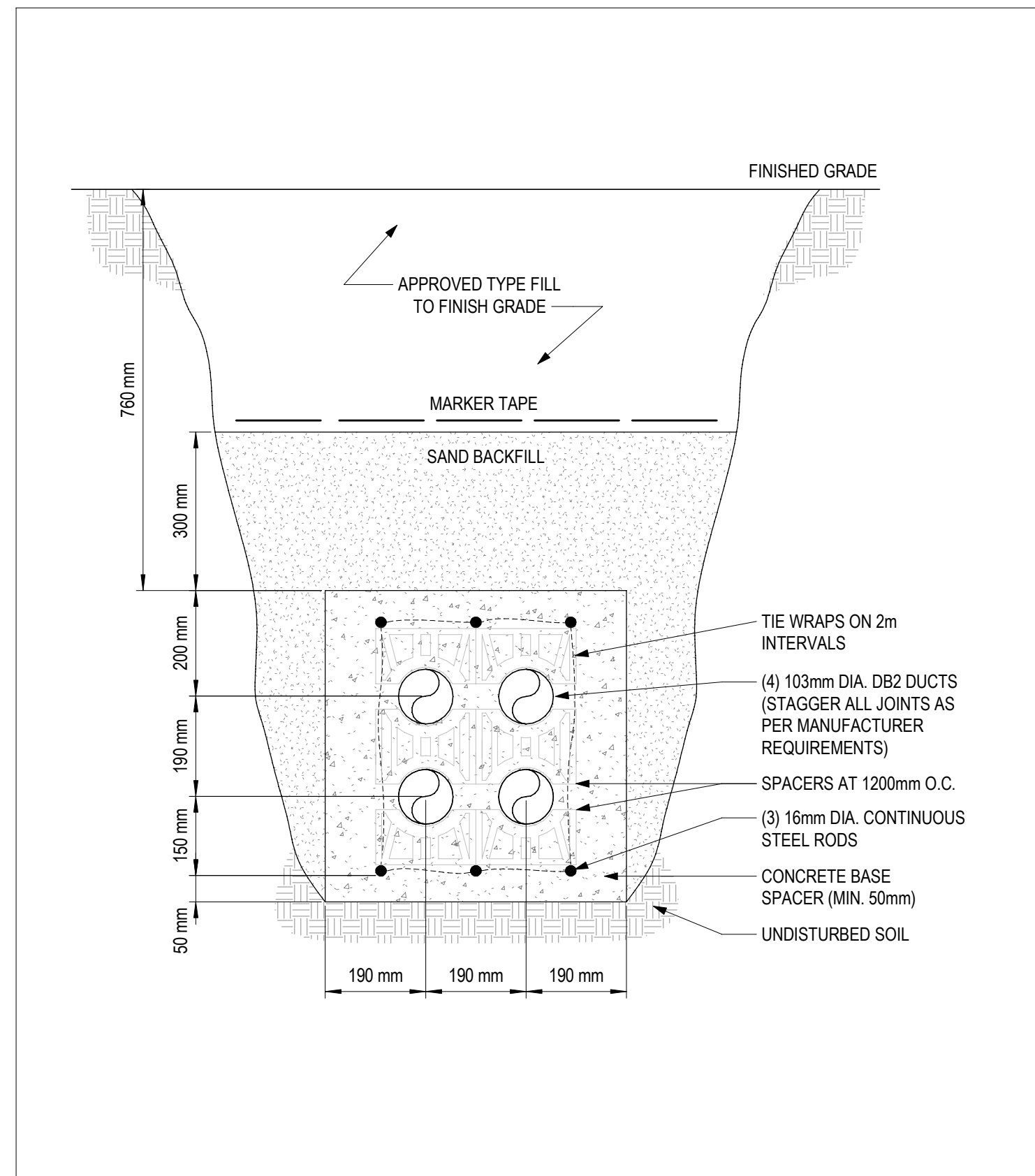
STRUCTURED CABLING

PUBLIC ADDRESS

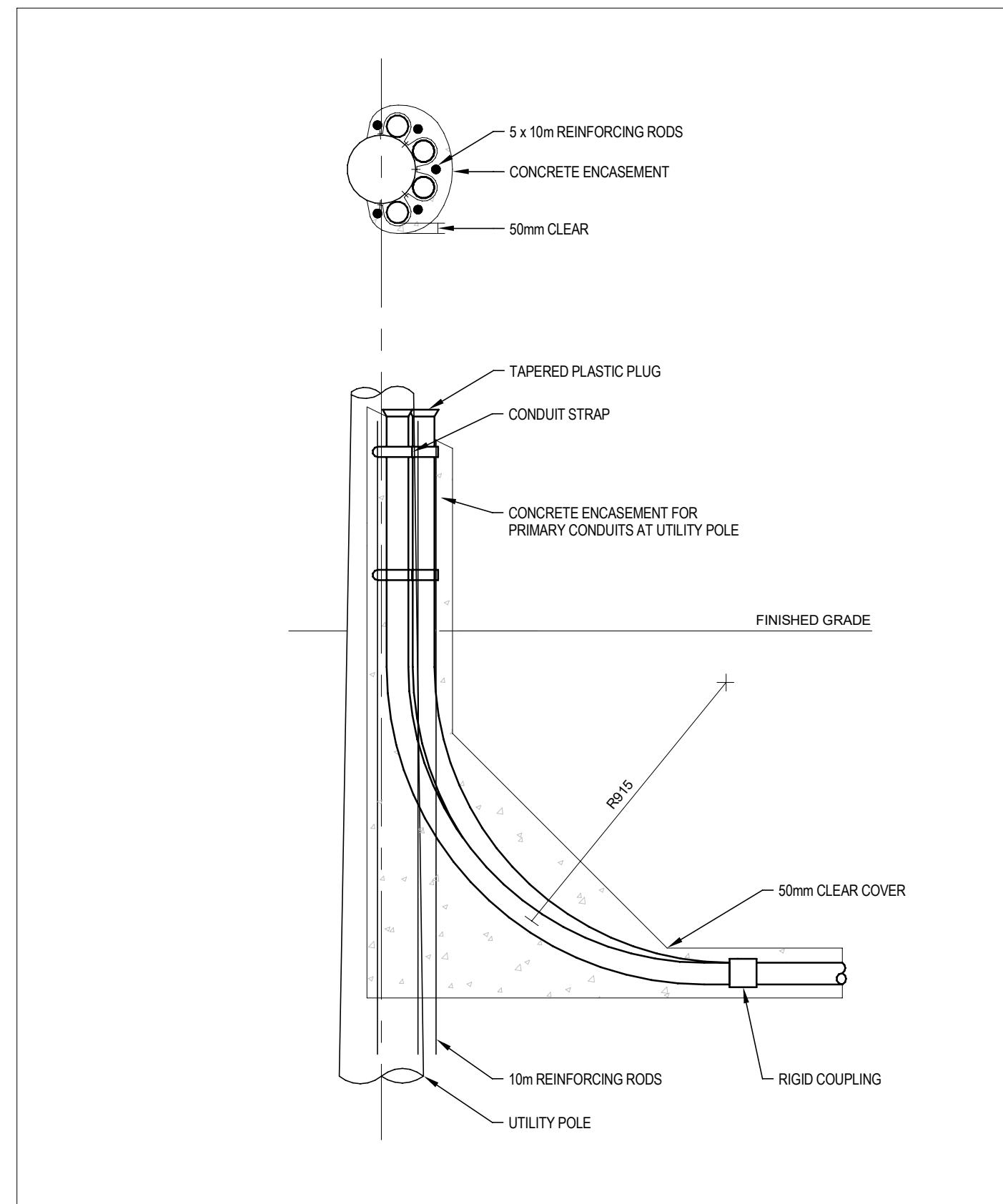
PLAN LIST			
PLAN NO.	DISCIPLINE	SUB-DISCIPLINE	PLAN NAME
E-100	ELECTRICAL	GENERAL	LEGEND, DRAWING LIST & KEY PLAN
E-110	ELECTRICAL	GENERAL	DIAGRAMS
E-111	ELECTRICAL	GENERAL	LIGHTING SCHEDULES
E-112	ELECTRICAL	GENERAL	SCHEDULES
E-113	ELECTRICAL	GENERAL	DETAILS
E-114	ELECTRICAL	GENERAL	DETAILS
E-200	ELECTRICAL	LIGHTING	GROUND FLOOR - DEMOLITION
E-201	ELECTRICAL	LIGHTING	GROUND FLOOR - NEW WORK
E-300	ELECTRICAL	POWER & SERVICES	GROUND FLOOR - DEMOLITION
E-301	ELECTRICAL	POWER & SERVICES	GROUND FLOOR - NEW WORK
E-302	ELECTRICAL	POWER & SERVICES	1ST FLOOR - NEW WORK
E-400	ELECTRICAL	FIRE ALARM	GROUND FLOOR - DEMOLITION
E-401	ELECTRICAL	FIRE ALARM	GROUND FLOOR - NEW WORK



ELECTRICAL - KEY PLAN
1 : 250



4 CELL CONCRETE ENCASED DUCT BANK DETAIL



HYDRO TERMINATION AT POLE

GENERAL NOTES

- A. PRIOR TO COMMENCING WORK, VERIFY THE LOCATION OF ALL BURIED SERVICES ON AND ADJACENT TO THE WORK AREA.
 - 1. THE CONTRACTOR IS RESPONSIBLE FOR SUBMITTING A LOCAL ONE-CALL/IN-CAUSE REQUEST TO THE UTILITY COMPANIES.
 - 2. THE CONTRACTOR SHALL OBTAIN A PRIVATE, INDEPENDENT UTILITY LOCATING SERVICE TO LOCATE ALL UTILITIES TO BE EXCAVATED.
 - 3. ELECTROMAGNETIC TRANSMISSION (EM) SIGNALS TO VERIFY INTERNAL, PRIVATELY OWNED UTILITIES SHALL BE OBTAINED FROM THE UTILITY COMPANIES.
 - 4. SUB-SURFACE UTILITY DATA DISCLOSED IN THESE PLANS ARE INFORMATIONAL ONLY. THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UTILITIES BY EXCAVATION, SURFACE AND VERTICAL POSITION, PLAN LOCATION OF ALL UTILITIES BY PLOT HOIST TESTING.
- B. PROTECT ALL EXISTING COULDS, ADJACENT, AND CROSSED UTILITIES PRIOR TO THE START OF ANY EXCAVATION OR CONSTRUCTION OPERATIONS.
- C. DO NOT LEAVE TRENCHES UNCOVERED. COVERS MUST BE SECURED AT THE END OF EVERY WORK DAY.
- D. PROVIDE TEMPORARY DRAINAGE PANS TO KEEP TRENCHES FREE FROM STANDING WATER.
- E. REMOVE AND RE-INSTALL FENCES OR OTHER SCHOOL, YARD EQUIPMENT AND REPAIR OR REPLACE ANY DAMAGE TO THE FENCES OR EQUIPMENT.
- F. PAY FOR ALL NEUTRAL FEES TO BE REINSTATED TO MATCH EXISTING UPON COMPLETION OF WORK.
- G. ALL TRENCHING AND EXCAVATIONS SHALL BE RESEALED USING HYDROVAC. SAVE ALL EXCAVATED MATERIALS AS REQUIRED AND REUSE (DO NOT DISPOSE) UNLESS OTHERWISE NOTED.

DRAWING NOTES

- [illegible]

COLOURS LEG
ELECTRICAL

PROJECT DRAWINGS MUST BE PRINTED IN COLOUR FOR CLARITY.

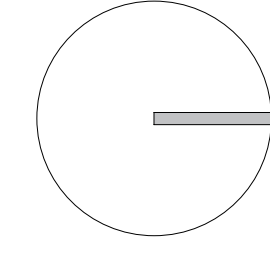
 NORMAL POWER
 GROUNDING/LIGHTNING PROTECTION
 DUCTBANK - ELECTRICAL
 EXISTING



SEALS



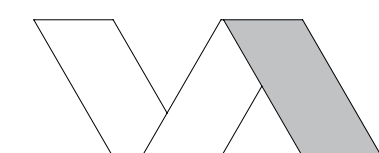
PROJECT NORTH



CLIENT



ARCHITEC



VEER ARCHITECTURE INC.
tel. 613-797-0047 email: info@veerarch.com
340 Gladstone Ave., Suite 301, Ottawa ON K2P 0Y7

THESE DOCUMENTS MUST NOT BE
USED FOR CONSTRUCTION

1	2026-07-30	ISSUED FOR ADDENDUM ELE-001	A.W.
0	2026-05-29	ISSUED FOR PERMIT AND TENDER	A.W.
No	DATE	REVISION	BY

PROJECT

ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUT
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

DRAWING TITLE

ELECTRICAL
GENERAL
LEGEND, DRAWING LIST & KEY
PLAN

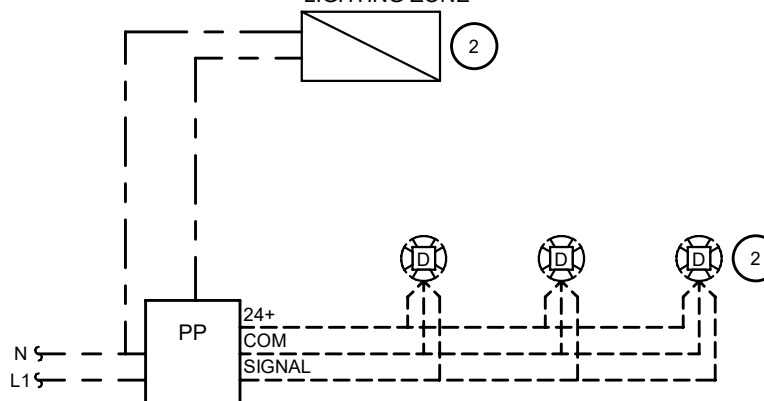
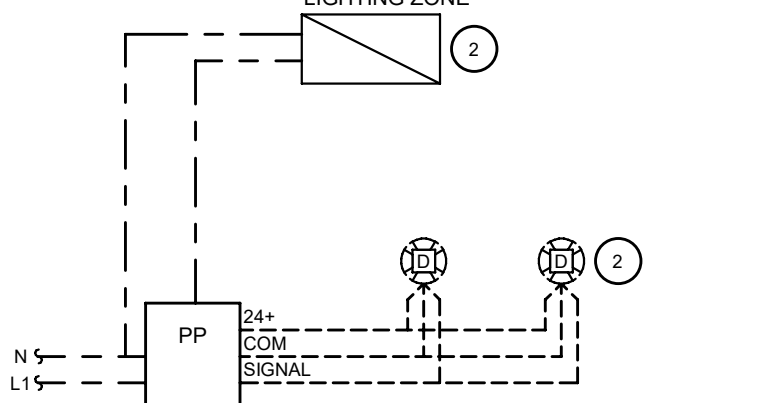
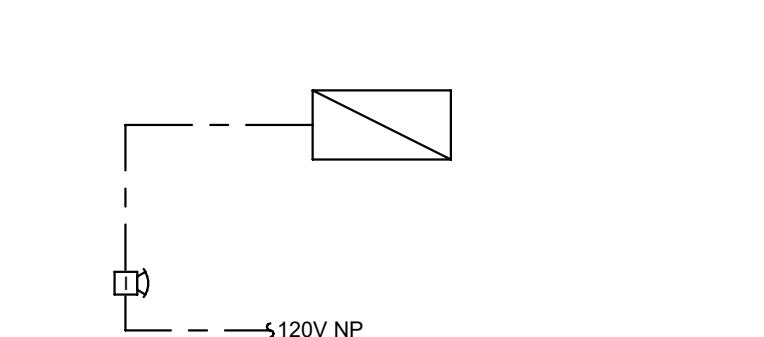
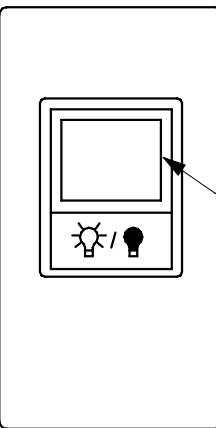
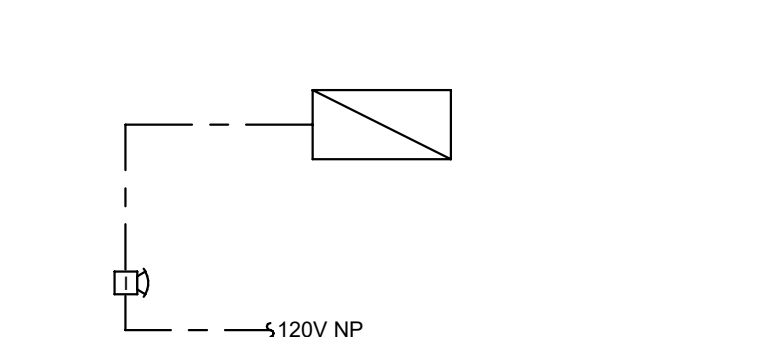
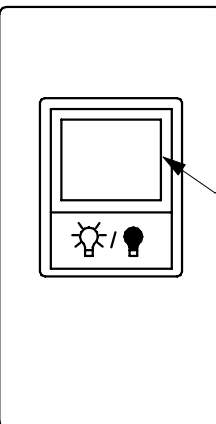
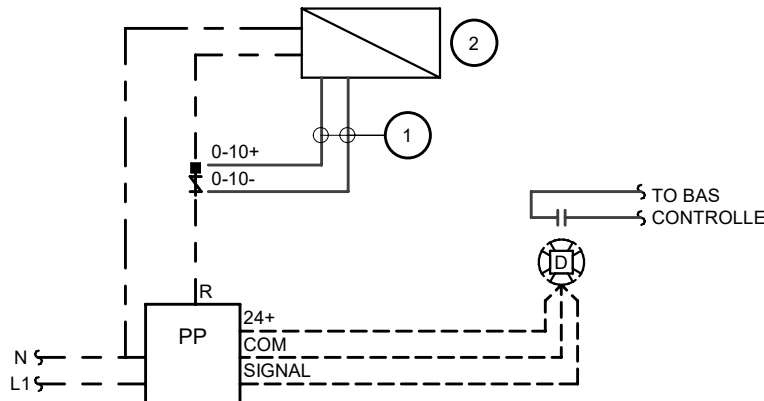
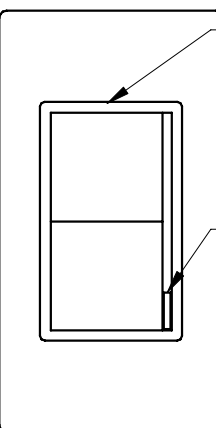
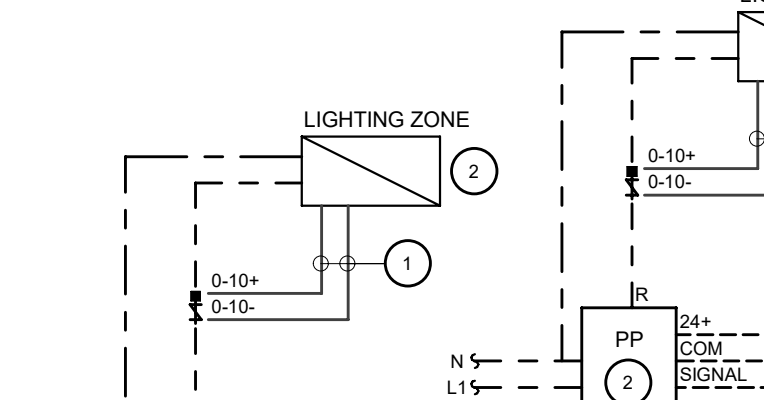
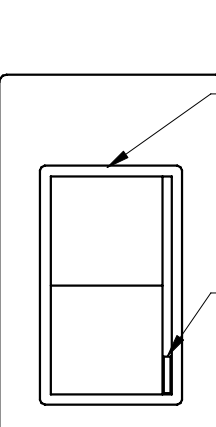
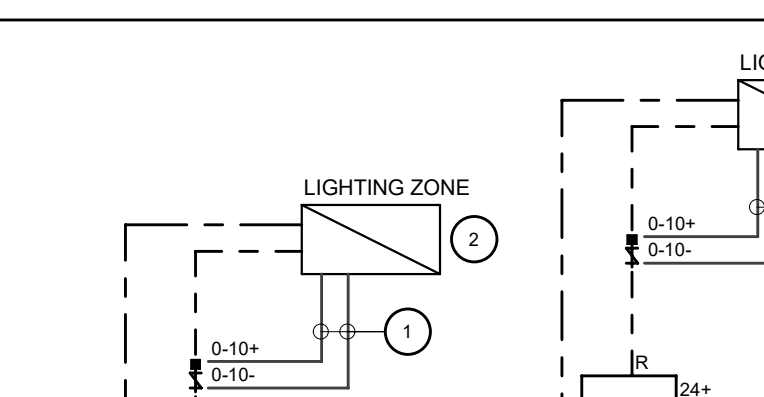
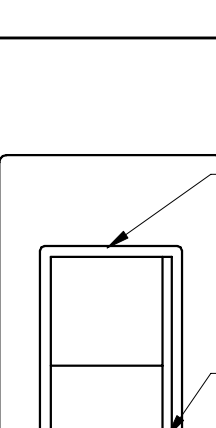
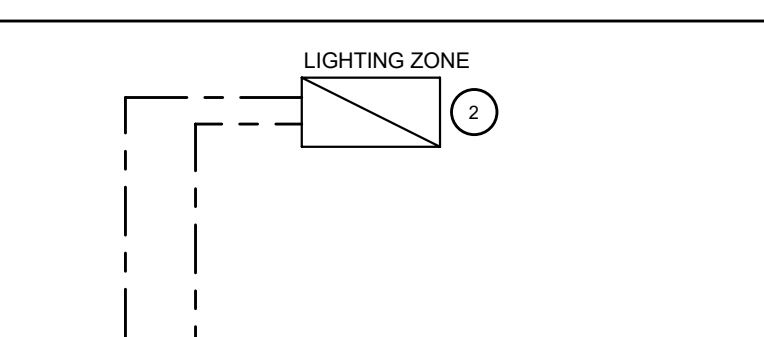
DRAWING N. LEMIEUX	DESIGN J. CICHOCKI	VERIFICATION A. WADDELL
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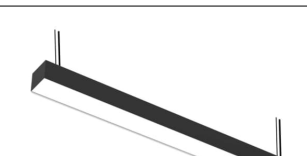
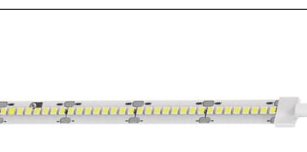
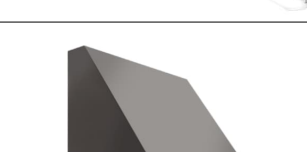
BPA PROJECT N

DRAWING No.

E-000

Autodesk Doc#:025-1159, Catholique, L.L. Couroux/025-1159, F. El. n.

LIGHTING CONTROLS FUNCTIONAL REQUIREMENTS SCHEDULE							
TYPE	AREA DESCRIPTION	DEVICE REQUIREMENTS		GENERAL OPERATION	SCHEMATIC	IDENTIFICATION	
	CORRIDORS	EM LIGHTING:	DOUBLE REMOTE HEADS FED FROM EMERGENCY BATTERY UNIT	BAS:	N/A		
		LIGHTING ZONES:	120VAC NORMAL POWER 24VDC EMERGENCY POWER	OCCUPANCY:	UPON OCCUPANT DETECTION LIGHTING IS TURNED ON AT 100%		
		LOCAL CONTROL:	N/A	VACANCY:	LIGHTING IS TURNED OFF AT 90% AFTER 20 MINUTE VACANCY PERIOD		
		OCCUPANCY SENSOR:	DUAL-TECH	LOCAL CONTROL:	N/A		
	LARGE WATERCLOSET	EM LIGHTING:	DOUBLE REMOTE HEADS FED FROM EMERGENCY BATTERY UNIT	BAS:	N/A		
		LIGHTING ZONES:	120VAC NORMAL POWER 24VDC EMERGENCY POWER	OCCUPANCY:	UPON OCCUPANT DETECTION LIGHTING IS TURNED ON AT 100%		
		LOCAL CONTROL:	N/A	VACANCY:	LIGHTING IS TURNED OFF AFTER 20 MINUTE VACANCY PERIOD		
		OCCUPANCY SENSOR:	DUAL-TECH	LOCAL CONTROL:	ON/OFF		
	SINGLE TOILET WATERCLOSET	EM LIGHTING:	DOUBLE REMOTE HEADS FED FROM EMERGENCY BATTERY UNIT	BAS:	N/A		
		LIGHTING ZONES:	1 NORMAL POWER	OCCUPANCY:	UPON OCCUPANT DETECTION LIGHTING IS TURNED ON AT 100%		
		LOCAL CONTROL:	TOGGLE SWITCH	VACANCY:	LIGHTING IS TURNED OFF AFTER 20 MINUTE VACANCY PERIOD		
		OCCUPANCY SENSOR:	INTEGRAL TO SWITCH, DUAL-TECH	LOCAL CONTROL:	ON/OFF		
	CLOSET & STORAGE ROOMS	EM LIGHTING:	EMERGENCY BATTERY UNIT AND REMOTE HEADS	BAS:	N/A		
		LIGHTING ZONES:	1 NORMAL POWER	OCCUPANCY:	UPON OCCUPANT DETECTION LIGHTING IS TURNED ON AT 100%		
		LOCAL CONTROL:	TOGGLE SWITCH	VACANCY:	LIGHTING IS TURNED OFF AFTER 20 MINUTE VACANCY PERIOD		
		OCCUPANCY SENSOR:	INTEGRAL TO SWITCH, DUAL-TECH	LOCAL CONTROL:	ON/OFF		
	STAFF ROOM	EM LIGHTING:	EMERGENCY BATTERY UNIT AND REMOTE HEADS	BAS:	N/A		
		LIGHTING ZONES:	1 NORMAL POWER	OCCUPANCY:	UNAFFECTED		
		LOCAL CONTROL:	TOGGLE SWITCH	VACANCY:	LIGHTING IS TURNED OFF AFTER 20 MINUTE VACANCY PERIOD		
		OCCUPANCY SENSOR:	DUAL-TECH	LOCAL CONTROL:	ON/OFF ± RAISE/LOWER		
	LARGE CLASSROOM	EM LIGHTING:	DOUBLE REMOTE HEADS FED FROM EMERGENCY BATTERY UNIT	BAS:	N/A		
		LIGHTING ZONES:	3-4 NORMAL POWER ZONES AS INDICATED ON PLANS, UNDERCABINET LIGHTING TO BE A ZONE	OCCUPANCY:	UNAFFECTED		
		LOCAL CONTROL:	TOGGLE SWITCH	VACANCY:	LIGHTING IS TURNED OFF AFTER 20 MINUTE VACANCY PERIOD		
		OCCUPANCY SENSOR:	DUAL-TECH	LOCAL CONTROL:	ON/OFF ± RAISE/LOWER		
	CLASSROOM	EM LIGHTING:	DOUBLE REMOTE HEADS FED FROM EMERGENCY BATTERY UNIT	BAS:	N/A		
		LIGHTING ZONES:	2 NORMAL POWER ZONES	OCCUPANCY:	UNAFFECTED		
		LOCAL CONTROL:	TOGGLE SWITCH	VACANCY:	LIGHTING IS TURNED OFF AFTER 20 MINUTE VACANCY PERIOD		
		OCCUPANCY SENSOR:	DUAL-TECH	LOCAL CONTROL:	ON/OFF ± RAISE/LOWER		
	EXTERIOR	EM LIGHTING:	N/A	BAS:	30 MINUTES BEFORE SUNSET = LIGHTS ON 30 MINUTES AFTER SUNRISE = LIGHTS OFF		
		LIGHTING ZONES:	NORMAL POWER	OCCUPANCY:	N/A		
		LOCAL CONTROL:	N/A	VACANCY:	N/A		
		OCCUPANCY SENSOR:	N/A	LOCAL CONTROL:	N/A		
WIRING LEGEND:		ABBREVIATIONS:		GENERAL DETAIL NOTES		DETAIL NOTES	
----- LIGHTING COMMUNICATION CABLE - COMMUNICATION CABLES TO BE AS PER LIGHTING CONTROL MANUFACTURER REQUIREMENTS - SUPPLIED AND INSTALLED BY ELECTRICAL CONTRACTOR		NP EMERGENCY POWER LCS LIGHTING CONTROL SYSTEM LC SWITCHING LIGHTING CONTROLLER 10VLC 0-10V LIGHTING LOAD CONTROLLER EPC EMERGENCY POWER CONTROL INTERFACE		A. AT TIME OF TENDER, OBTAIN MANUFACTURERS SPECIFIC INSTALLATION INSTRUCTIONS, SCHEMATICS AND DIAGRAMS FROM LIGHTING CONTROL VENDOR. ALLOW FOR ALL LABOR AND MATERIALS REQUIRED FOR THE COMPLETE AND FUNCTIONAL INSTALLATION OF THE LIGHTING CONTROL SYSTEM. B. PERFORM INSTALLATION OF LIGHTING CONTROL SYSTEM IN CONFORMANCE WITH MANUFACTURER INSTALLATION INSTRUCTION. PROVIDE ANY LIGHTING CONTROL AUXILIARY DEVICES REQUIRED FOR A COMPLETE AND FUNCTIONAL INSTALLATION. C. PROVIDE DEDICATED 21 mm CONDUIT FOR CONTROL CABLEING BETWEEN LIGHTING LOAD CONTROLLERS AND LUMINAIRES FOR INACCESSIBLE AND EXPOSED CEILING. D. TO IDENTIFY LIGHTING CONTROL FUNCTIONALITY, PROVIDE CUSTOM ENGRAVINGS FOR DEVICES & FACE PLATES AS SHOWN IN THESE DETAILS. ALL BUTTONS TO BE ENGRAVED POST COMPLETION AND ACCEPTANCE OF SYSTEM DEMONSTRATION. SUBMIT PROPOSED FACE PLATE AND BUTTON ENGRAVINGS TO OWNER REPRESENTATIVE FOR REVIEW. E. WALL STATION FACE PLATES TO BE SCREWLESS TYPE AND HAVE NO VISIBLE HARDWARE. THE FRONT PLATE SECURELY SNAPS INTO THE ALIGNMENT ADAPTER PLATE. F. THESE DETAILS SHOWS THE GENERAL CONFIGURATION OF DEVICES AND FACE PLATES. REFER TO 200 SERIES DRAWINGS FOR DEVICES QUANTITIES AND GANGING.		1 PROVIDE 0-10V CONTROL CABLEING BETWEEN LUMINAIRE AND DIMMER SWITCH. CABLEING TO BE COORDINATED WITH MANUFACTURER REQUIREMENTS. 2 REFER TO 200 SERIES DRAWINGS FOR QUANTITIES AND LOCATIONS OF LIGHTING AND CONTROL DEVICES. 3 OCCUPANCY SENSORS AUXILIARY CONTACT TO BAS CONTROLLER. REFER TO M-800 SERIES. 4 POWER PACK TO BE CONTROLLED BY BAS. SEQUENCE OF OPERATION TO BE TIME OF DAY BASED.	
----- #12-2C 120V CABLEING - SUPPLIED AND INSTALLED BY ELECTRICAL CONTRACTOR							
----- OTHER - AS INDICATED ON DETAILS							

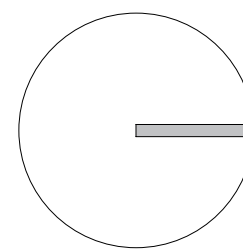
LIGHTING FIXTURE AND EXIT SIGNAGE SCHEDULE													
TYPE	DESCRIPTION	MATERIALS	LIGHT SOURCE			VOLTAGE	WATTAGE	DIMMING	FINISH	MOUNTING	NOTES		
			TYPE	LUMENS	CRI								
RT1	2' x 4' SHALLOW TROFFER		LED	4,000	80 +	3500 K	120-277 VAC	35 W	0-10V	WHITE	RECESSED T-BAR	- ADJUSTABLE LUMEN OUTPUT 4000/5000/6000 - SWITCHABLE CCT 3500K/4000K/5000K	
RT2	2' x 4' SHALLOW TROFFER		LED	5,000	80 +	3500 K	120-277 VAC	45 W	0-10V	WHITE	RECESSED T-BAR	- ADJUSTABLE LUMEN OUTPUT 4000/5000/6000 - SWITCHABLE CCT 3500K/4000K/5000K	
R1	4" ROUND DOWNLIGHT		LED	750	80 +	3500 K	120-277 VAC	9 W	0-10V	WHITE	RECESSED T-BAR	- ADJUSTABLE LUMEN OUTPUT 500/750/1000 - SWITCHABLE CCT 3000K/3500K/4000K/5000K - WIDE ANGLE DISTRIBUTION	
R2	4" ROUND DOWNLIGHT		LED	1,000	80 +	3500 K	120-277 VAC	13 W	0-10V	WHITE	RECESSED T-BAR	- ADJUSTABLE LUMEN OUTPUT 1000/1500/2000 - SWITCHABLE CCT 3000K/3500K/4000K/5000K - WIDE ANGLE DISTRIBUTION	
R3	4" ROUND DOWNLIGHT		LED	1,000	80 +	3500 K	120-277 VAC	13 W	0-10V	WHITE	RECESSED GYPSUM	- ADJUSTABLE LUMEN OUTPUT 1000/1500/2000 - SWITCHABLE CCT 3000K/3500K/4000K/5000K - WIDE ANGLE DISTRIBUTION	
S1	4' SUSPENDED LINEAR FIXTURE		LED	500/ft.	80 +	3500 K	120-277 VAC	16 W	0-10V	TBD	SUSPENDED	- 4" WIDE FIXTURE TO BE SUSENDED IN WOODEN SLAT CEILING, COORDINATE INSTALLATION	
L1	LINEAR TAPE STRIP LIGHT		LED	300/ft.	90 +	3500 K	24 V	2.5 W/FT	N/A	WHITE	UNDER CABINET U-CLIP	- PROVIDE ALL ACCESSORIES REQUIRED FOR A COMPLETE INSTALLATION - STRIP TO HAVE CHANNEL/FROSTED COVER - DRIVER TO BE LOCATED OUT OF SIGHT	
L2	4" SURFACE MOUNT STRIP LIGHT		LED	4,000	80 +	3500 K	120 VAC	30 W	N/A	WHITE	SURFACE	- ADJUSTABLE LUMEN OUTPUT 3000/4000/5000 - SWITCHABLE CCT 3500K/4000K/5000K	
W1	EXTERIOR WALL PACK		LED	3,200	80 +	3500 K	120 VAC	23 W	N/A	TBD	WALL MOUNTED		
RH1	DOUBLE EMERGENCY REMOTE HEAD		LED	N/A	N/A	N/A	24 V	4 W	N/A	WHITE	WALL MOUNTED	- TWO (2) HEADS PER CANOPY	
EBU1 TO EBU3	STEEL BATTERY UNIT WITH DOUBLE EMERGENCY REMOTE HEAD		LED	N/A	N/A	N/A	120 VAC/ 24 V	100 W	N/A	WHITE	WALL MOUNTED	- TWO (2) HEADS - AUTOMATIC TEST SELF-DIAGNOSTIC - WATTAGE CAPACITY SUITABLE FOR MINIMUM OF 100W LOAD AT 90 MINUTE RUN-TIME	
EBU4	STEEL BATTERY UNIT WITH DOUBLE EMERGENCY REMOTE HEAD		LED	N/A	N/A	N/A	120 VAC/ 24 V	30 W	N/A	WHITE	WALL MOUNTED	- TWO (2) HEADS - AUTOMATIC TEST SELF-DIAGNOSTIC - WATTAGE CAPACITY SUITABLE FOR MINIMUM OF 30W LOAD AT 90 MINUTE RUN-TIME	
X1	RUNNING MAN EXIT SIGN		LED	N/A	N/A	N/A	120 VAC/ 24 V	2 W	N/A	WHITE	SURFACE/ RECESSED AS REQUIRED	- UNIVERSAL FACE AND PICTOGRAM - SINGLE OR DOUBLE SIDED - CSA C22.2 NO. 141 CERTIFIED	



SEALS



PROJECT NORTH



CLIENT



ARCHITECT

THESE DOCUMENTS MUST NOT BE
USED FOR CONSTRUCTION

1	2026-07-30	ISSUED FOR ADDENDUM ELE-001	A.W.	
0	2026-05-29	ISSUED FOR PERMIT AND TENDER	A.W.	
No	DATE	REVISION	A.W.	BY

PROJECT

ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUX
10 FINDLAY AVE
CARLETON PLACE
2025JLCL117

DRAWING TITLE

ELECTRICAL
GENERAL
LIGHTING SCHEDULES

DRAWING N. LEMIEUX	DESIGN J. CICHOCKI	VERIFICATION A. WADDELL
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BPA PROJECT No

25-1159

SCALE

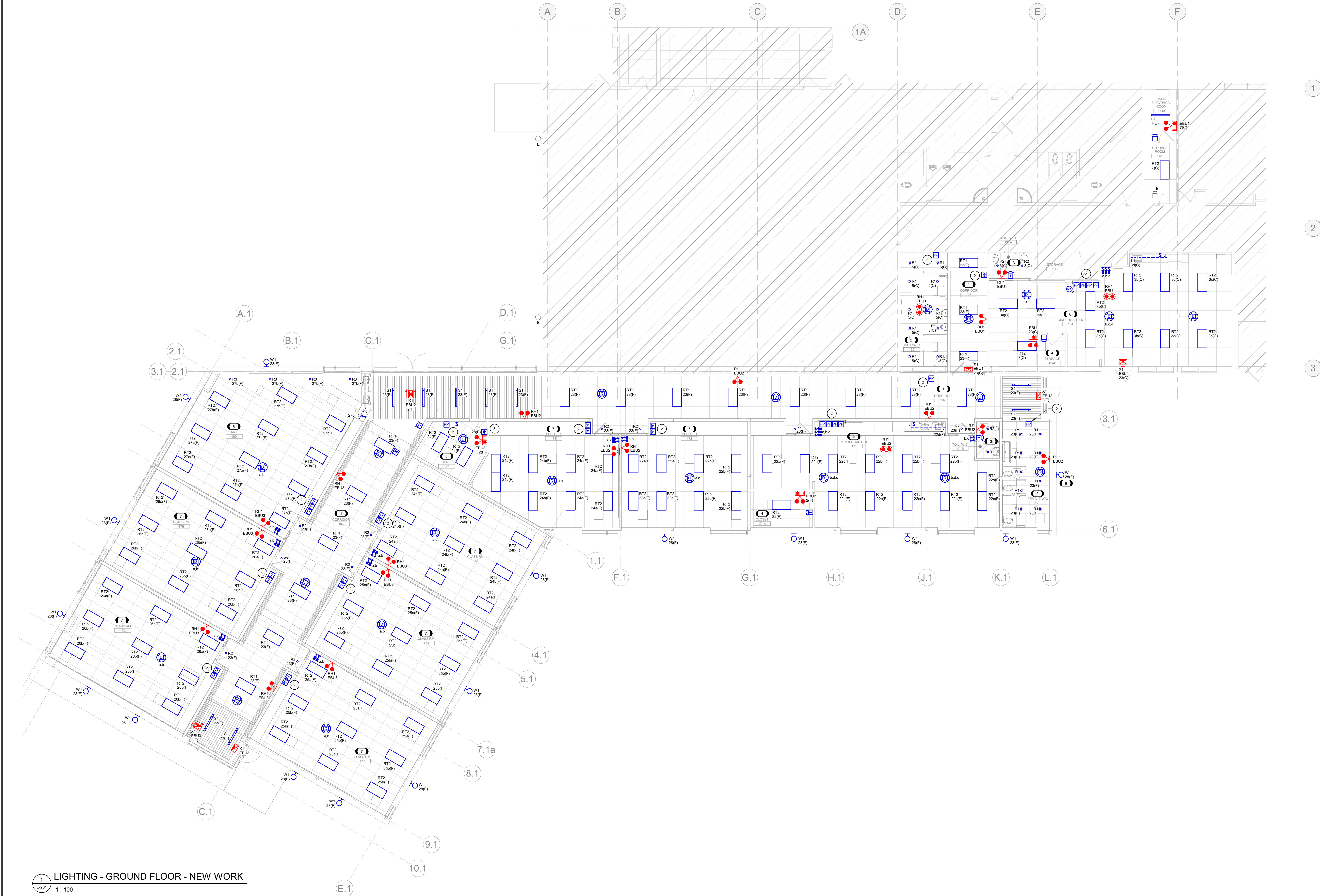
NTS

E-101

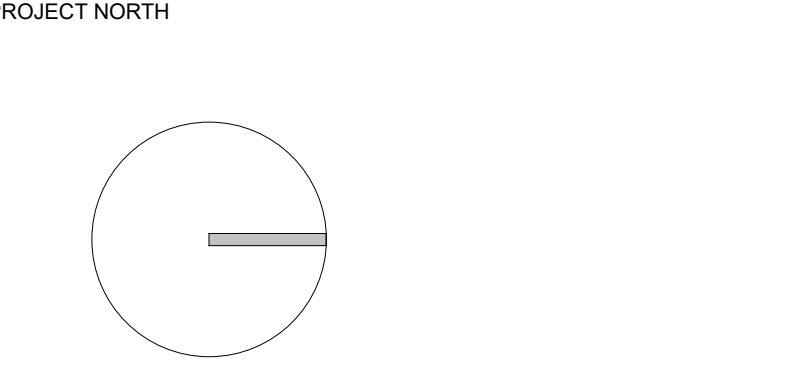
EXISTING PANEL B(B)														
Room: DP-01 Feeder: DP-01 Level: Lo. SLAB				Voltage: 120/208V Phases: 3 Wires: 4 AIC:				Bus Material: Bus Ampacity: 225 A Main Circuit Breaker: N/A						
Note	Description	Br.	Pole s	CC T	A	B	C	CC T	Pole s	Br.	Description	Note		
	EXISTING	15 A	1	1	0 W	0 W			2	1	15 A	EXISTING		
	EXISTING	15 A	1	3		0 W	0 W		4	1	15 A	EXISTING		
	EXISTING	15 A	1	5				0 W	6	1	15 A	EXISTING		
	EXISTING	15 A	1	7	0 W	0 W			8	1	15 A	EXISTING		
	EXISTING	15 A	1	9		0 W	0 W		10	1	15 A	EXISTING		
	EXISTING	15 A	1	11			0 W	0 W	12	1	15 A	EXISTING		
	EXISTING	15 A	1	13	0 W	0 W			14	1	15 A	EXISTING		
	EXISTING	15 A	1	15		0 W	0 W		16	1	15 A	EXISTING		
	EXISTING	15 A	1	17			0 W	0 W	18	1	15 A	EXISTING		
	EXISTING	15 A	1	19	0 W	0 W			20	1	15 A	EXISTING		
	EXISTING	15 A	1	21		0 W	0 W		22	1	15 A	EXISTING		
	EXISTING	15 A	1	23			0 W	0 W	24	1	15 A	EXISTING		
	EXISTING	15 A	1	25	0 W	0 W			26	1	20 A	EXISTING		
	EXISTING	15 A	1	27		0 W	0 W		28	1	15 A	EXISTING		
	EXISTING	20 A	1	29				0 W	0 W	30	1	15 A	EXISTING	
	EXISTING	20 A	1	31	0 W	0 W			32	1	15 A	EXISTING		
	EXISTING	15 A	1	33		0 W	0 W		34	1	15 A	EXISTING		
	EXISTING	15 A	1	35			0 W	0 W	36	1	15 A	EXISTING		
	EXISTING	15 A	1	37	0 W	0 W			38	1	20 A	EXISTING		
	EXISTING	15 A	1	39		0 W	0 W		40	2	40 A	EXISTING		
	EXISTING	20 A	2	41				0 W	000 W	42	1	15 A	HWT-Q1 3	
	EXISTING	15 A	1	43	0 W	0 W			44	1	15 A	HWT-Q2 3		
	EXISTING	15 A	1	45		0 W	0 W		46					
	EXISTING	15 A	1	47			0 W	0 W	48	3	30 A	EXISTING		
	EXISTING	20 A	2	49	0 W	0 W			50					
	EXISTING	15 A	1	51		0 W	0 W		52	1	15 A	EXISTING		
	EXISTING	15 A	1	53			0 W	0 W	54	1	15 A	EXISTING		
	EXISTING	15 A	1	55	0 W	0 W			56	1	15 A	EXISTING		
	EXISTING	15 A	1	57		0 W	0 W		58	1	15 A	EXISTING		
	EXISTING	15 A	1	59			0 W	0 W	60	1	15 A	EXISTING		
	EXISTING	15 A	1	61	0 W	0 W			62	1	15 A	EXISTING		
	EXISTING	15 A	1	63		0 W	0 W		64	1	15 A	EXISTING		
	EXISTING	15 A	1	65			0 W	0 W	66	1	15 A	EXISTING		
1	HOUSEKEEPING RCPT	20 A	1	67	0 W	0 W	300 W		68	1	15 A	EXISTING		
2	HAND DRYER W/C 135	20 A	1	69			1000 W		70	1	15 A	EXISTING		
2	HAND DRYER W/C 135	20 A	1	71				1000 W	72	1	20 A	EXISTING		
Total Charge:				1040 W				1000 W				1500 W		
Total Courant :				9 A				8 A				13 A		
Panel Summary														
Total Connected Load:												3540 W		
Total Connected Amperage:												10 A		
Notes:														
0. CUTLER-HAMMER POW-R-LINE C PRL-1 PANELBOARD														
1. PROVIDE NEW BREAKER														
2. PROVIDE NEW GFCI TYPE CIRCUIT BREAKER														
3. REMOVE EXISTING 20A, 2P BREAKER FOR ELECTRIC FORCE FLOW HEATER EFF-5 DEMOLISHED AS PART OF THESE RENOVATIONS. PROVIDE TWO NEW 15A, 1P BREAKERS FOR HOT WATER TANK CONNECTIONS.														

EXISTING PANEL C(C)													
Room: Feeder: DP-01 Level: L.o. SLAB				Voltage: 120/208V Phases: 3 Wires: 4 AIC:				Bus Material: Bus Ampacity: 225 A Main Circuit Breaker: N/A					
Note	Description	Br.	Pole s	CC T	A	B	C	CC T	Pole s	Br.	Description	Note	
1	EXISTING	15 A	1	1	0 W	0 W			2	1	15 A	EXISTING	
1	LGT - EXISTING + KINDERGARTEN 134	15 A	1	3			0 W	0 W	4	1	15 A	EXISTING	
1	LGT - EXISTING + RMS 106 & 135	15 A	1	5					6	2	15 A	EXISTING	
1	LGT - EXISTING + RMS 141 & 141a	15 A	1	7	200 W	0 W		130 W	0 W	8	2	15 A	EXISTING
	EXISTING	15 A	1	9			0 W	0 W	10	1	15 A	EXISTING	
	EXISTING	20 A	1	11				0 W	0 W	12	1	15 A	EXISTING
	EXISTING	20 A	1	13	0 W	0 W				14	1	15 A	EXISTING
	EXISTING	15 A	1	15			0 W	0 W		16	2	15 A	EXISTING
	EXISTING	15 A	1	17				0 W	0 W	18	2	15 A	EXISTING
	EXISTING	15 A	1	19	0 W	0 W				20	1	15 A	EXISTING
	EXISTING	15 A	1	21			0 W	0 W		22	1	15 A	EXISTING
1	EXISTING + NEW EXITS & EBU	15 A	1	23			140 W	0 W		24	1	15 A	EXISTING
	EXISTING	15 A	1	25	0 W	0 W				26	1	15 A	EXISTING
	EXISTING	15 A	1	27			0 W	0 W		28	1	15 A	EXISTING
	EXISTING	20 A	2	29				0 W	0 W	30	2	15 A	EXISTING
	EXISTING	15 A	2	31	0 W	0 W				32	2	15 A	EXISTING
	EXISTING	15 A	1	33			0 W	0 W		34	1	15 A	EXISTING
	EXISTING	15 A	1	35				0 W	0 W	36	1	15 A	EXISTING
	EXISTING	15 A	1	37	0 W	0 W				38	1	15 A	EXISTING
	EXISTING	15 A	2	39			0 W	0 W		40	1	15 A	EXISTING
	EXISTING	15 A	2	41				0 W	0 W	42	1	15 A	EXISTING
	EXISTING	15 A	2	43	0 W	0 W				44	1	15 A	EXISTING
	EXISTING	15 A	2	45			0 W	0 W		46	2	50 A	EXISTING
	EXISTING	15 A	2	47				0 W	0 W	48	2	50 A	EXISTING
	EXISTING	15 A	2	49	0 W	0 W				50	1	15 A	EXISTING
	EXISTING	15 A	1	51			0 W	0 W		52	1	15 A	EXISTING
3	EF-03	15 A	1	53				70 W	0 W	54	1	15 A	EXISTING
	EXISTING	15 A	1	55	0 W	0 W				56			
	EXISTING	15 A	1	57			0 W	0 W		58	3	15 A	EXISTING
	IF-3	15 A	1	59				800 W	0 W	60			
	EXISTING	15 A	1	61	0 W	0 W				62	1	15 A	EXISTING
	EXISTING	15 A	1	63			0 W	0 W		64	1	15 A	EXISTING
	EXISTING	20 A	2	65				0 W	0 W	66	2	15 A	EXISTING
	EXISTING	20 A	2	67	0 W	0 W				68			
	EXISTING	20 A	2	69			0 W	0 W		70			
	EXISTING	20 A	2	71				0 W	0 W	72	2	20 A	EXISTING
Total Charge:				339 W				1214 W				953 W	
Total Courant :				3 A				11 A				9 A	
Panel Summary													
Total Connected Load: 2506 W													
Total Connected Amperage: 7 A													
Notes:													
0. CUTLER-HAMMER POW-R-LINE C PRL-1 PANELBOARD													
1. CONTRACTOR TO CONFIRM EXISTING CIRCUIT AND LOAD. RE-USE EXISTING CIRCUITS IN RENOVATED AREAS FOR NEW LIGHTING. UPDATE PANEL SCHEDULES													
2. WATER HEATER REMOVED FROM CIRCUIT. PROVIDE CONNECTION TO NEW RECIRCULATION PUMP. UPDATE PANEL SCHEDULE.													
3. RECONNECT TO NEW EXHAUST FAN UPON COMPLETION OF MECHANICAL INSTALLATION.													

BP-F(F)													
Room: STAFF 174 Feeder: 1 Level: L.o. SLAB					Voltage: 120/208V Phases: 3 Wires: 4 AIC: 18kA					Bus Material: COPPER Bus Ampacity: 225 A Main Circuit Breaker: N/A			
Note	Description	Br.	Pole s	CC T	A	B	C	CC T	Pole s	Br.	Description	Note	
	ROOFTOP MAINTENANCE RCPT	20 A	1	1	300 W	240 W		2	1	15 A	BATTERY PACKS AND EXIT SIGNS		
	HOUSEKEEPING RCPT	20 A	1	1	300 W	900 W	1000 W		4	1	20 A	HAND DRYER W/C 170	1
	HOUSEKEEPING RCPT	20 A	1	1	300 W			1000 W	6	1	20 A	HAND DRYER W/C 170	1
	HOUSEKEEPING RCPT	20 A	1	1	300 W				8				
1	BOTTLE FILLING STATION	15 A	1	9		180 W	1080 W		10	1	15 A	RECEPTACLES RM 171 & 172	
	RECEPTACLES RM 171 & 172	15 A	1	11			1000 W	1000 W	12	1	15 A	RECEPTACLES RM 173 & 175	
	RECEPTACLES RM 174 & 180	15 A	1	13	900 W	720 W			14	1	15 A	RECEPTACLES RM 173 & 175	
	RECEPTACLES RM 176 & 177	15 A	1	15		800 W	800 W		16	1	15 A	RECEPTACLES RM 176 & 177	
	RECEPTACLES RM 178 & 179	15 A	1	17			800 W	800 W	18	1	15 A	RECEPTACLES RM 178 & 179	
	RECEPTACLES RM 180	15 A	1	19	720 W	180 W			20	1	20 A	IT CABINET	
	SECURITY CONTROL PANEL	20 A	1	21		600 W	1174 W		22	1	15 A	LGT - RM 171-172	
	LGT - CORRIDOR 107 & W/C 170	15 A	1	23			1440 W	1000 W	24	1	15 A	LGT - RMS 173-175	
	LGT - RM 176-177	15 A	1	25	990 W	990 W			26	1	15 A	LGT - RM 178-179	
	LGT - RM 180	15 A	1	27		720 W	360 W		28	1	15 A	LGT - EXTERIOR	
1	SP-01	15 A	1	29				220 W	30				
				31	1000 W				32				
	CUH-01	15 A	3	33		1000 W			34				
				35				1000 W	36				
				37	1000 W				38				
	CUH-02	15 A	3	39		1000 W			40				
				41				1000 W	42				
				43	3000 W				44				
	CUH-03	35 A	3	45		3000 W			46				
				47			3000 W		48				
	EF-08	15 A	1	49	75 W				50				
	PLUMBING FIXTURES	15 A	1	51		200 W			52				
				53					54				
				55					56				
				57					58				
				59					60				
				61					62				
				63					64				
				65					66				
				67	0 W				68	1	15 A	SPARE	
	SPARE	15 A	1	69		0 W	0 W		70	1	15 A	SPARE	
	SPARE	15 A	1	71			0 W	0 W	72	1	20 A	SPARE	
Total Charge:					10864 W	13445 W	13009 W						
Total Countour :					91 A	115 A	111 A						
Panel Summary													
					Total Connected Load: 37319 W								
					Total Connected Amperage: 104 A								
Notes:													
1. GFCI TYPE CIRCUIT BREAKER													



- GENERAL NOTES**
- A. (C) INDICATES THE FUNCTIONAL LIGHTING CONTROL REQUIREMENTS. REFER TO SCHEDULE ON DRAWING E-01.
 - B. MOUNT LIGHTING CONTROLLERS ABOVE ACCESSIBLE CEILING.
 - C. ALL DEVICES ON THIS PLAN ARE NEW UNLESS OTHERWISE NOTED. REFER TO LIGHTING FUTURE SCHEDULE ON DRAWING E-01.
 - D. PROVIDE ALL NEW WIRING IN EMT. NO MORE THAN 1500mm OF FLEX/AC-90 SHALL BE PERMITTED.
 - E. ALL FIXTURES IN 1' DIA CEILING MUST HAVE 4 FIXTURE CHAINS TO BUILDING STRUCTURE. USE STEEL 'S' HOOKS WITH CHAINS. REFER TO DETAIL 3-ONE-103.
 - F. LIGHTING ZONE(S) ARE DEFINED WITHIN ROOM AREAS.
 - G. EXISTING LIGHTING CIRCUIT INFORMATION FROM AS-BUILTS. CONTRACTOR TO CONFIRM AVAILABLE CIRCUITS AND CAPACITY. CONTRACTOR SHALL REPORT FINDINGS TO CONSULTANT AHEAD OF SHOP DRAWINGS AND ORDERING MATERIALS. LIGHTING IN RENOVATED PORTION OF BUILDING TO REUSE EXISTING CIRCUITS.
- DRAWING NOTES**
- 1. COORDINATE EXTERIOR WALL PACKS WITH ARCHITECTURAL ELEVATIONS.
 - 2. PROVIDE POWER PACK FOR EACH LIGHTING ZONE IN ACCESSIBLE LOCATION ABOVE CEILING.
 - 3. DIGITAL TIME CLOCKS FOR CONTROL OF CORRIDOR AND EXTERIOR LIGHTS. PROVIDE LOCAL OR SYSTEM CONTROLLERS TO ACHIEVE FUNCTIONALITY DESCRIBED IN THE LIGHTING CONTROLS FUNCTIONAL REQUIREMENTS SCHEDULE.



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1	2026-07-30	ISSUED FOR ADDENDUM ELE-001	A.W.
0	2026-05-29	ISSUED FOR PERMIT AND TENDER	A.W.

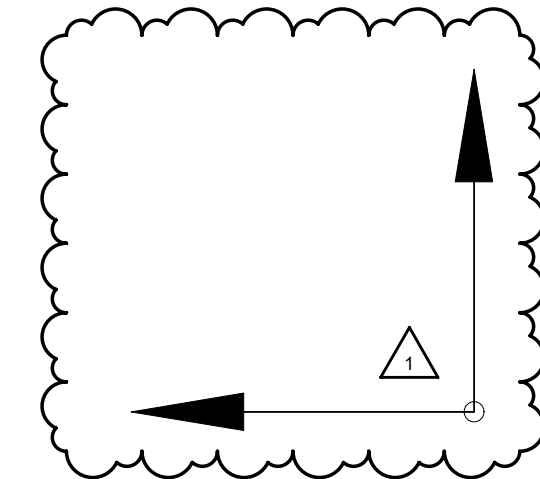
PROJECT
ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUX
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

DRAWING TITLE
ELECTRICAL
LIGHTING
GROUND FLOOR
NEW WORK

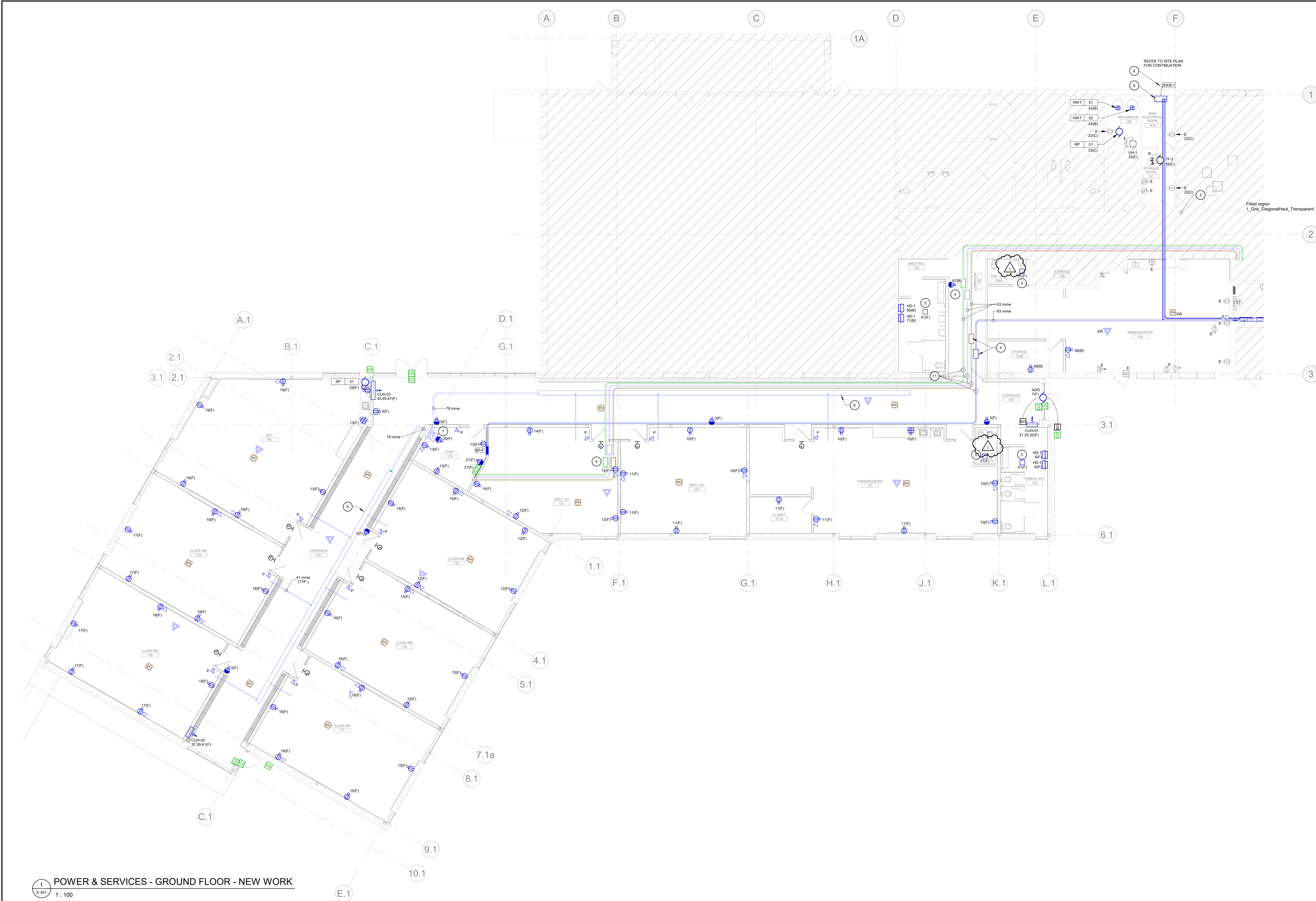
DRAWING	DESIGN	VERIFICATION
N. LEMIEUX	J. CICHOCKI	A. WADDELL

BPA PROJECT No
25-1159
SCALE
NTS

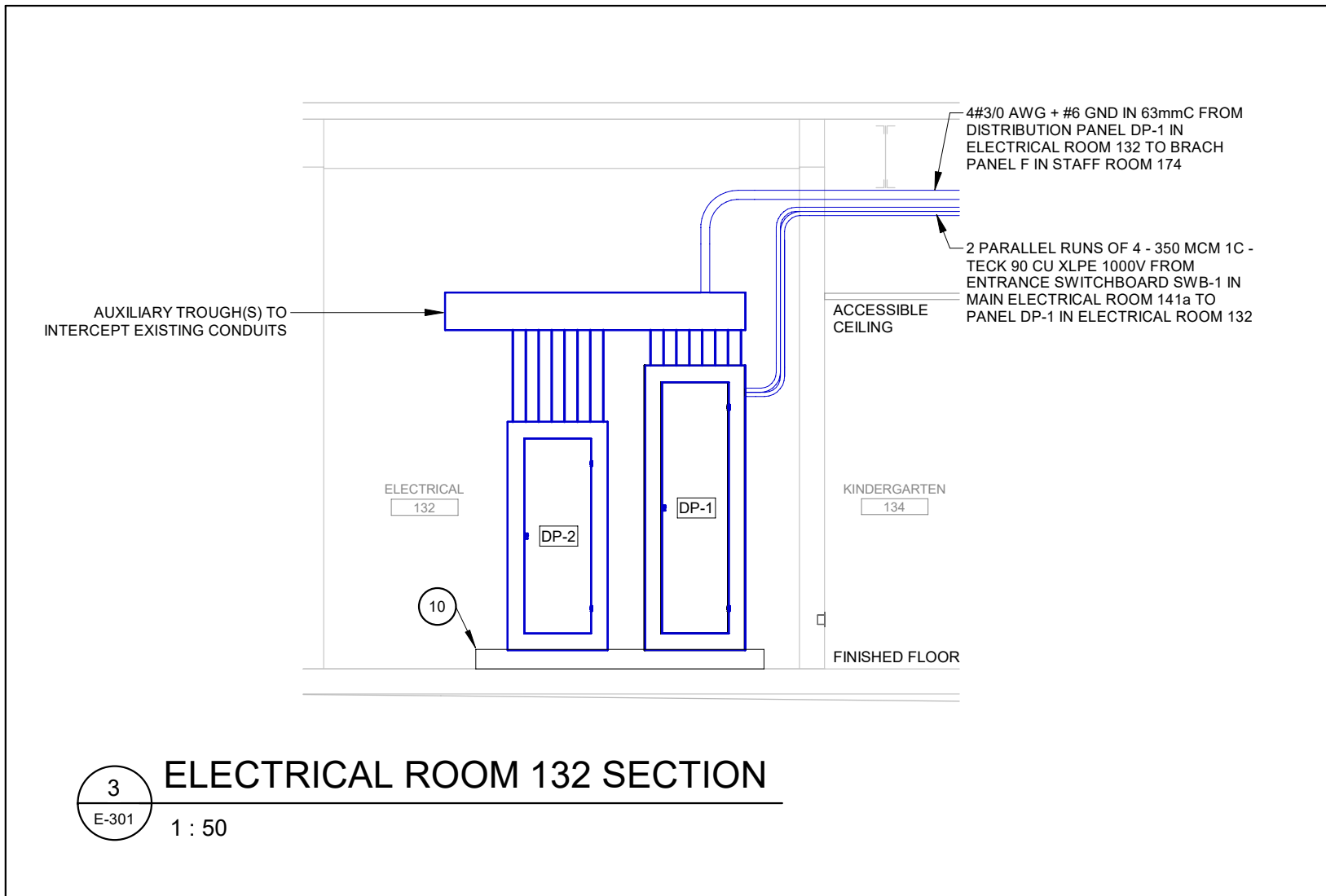
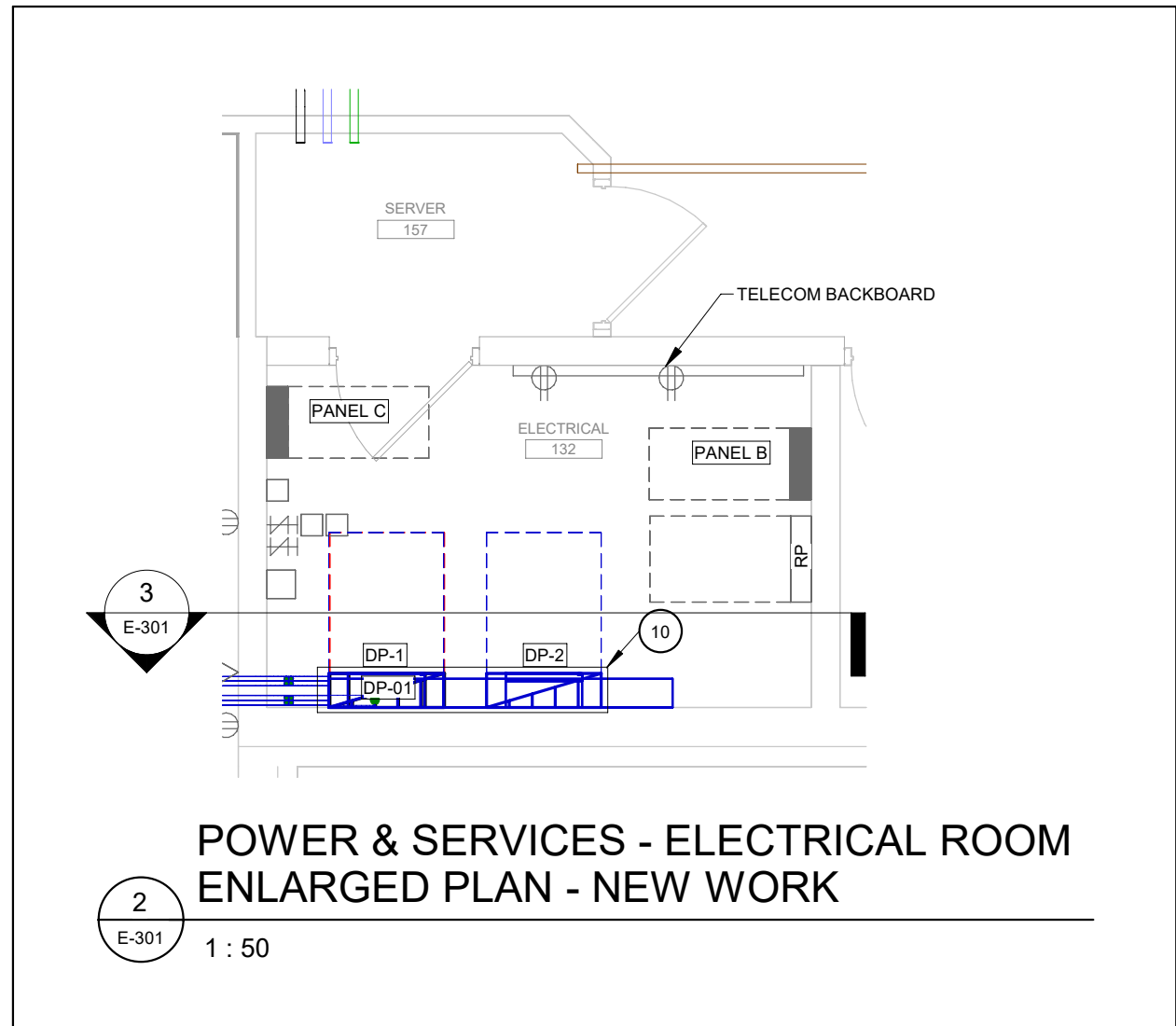
E-201



COLOURS LEGEND ELECTRICAL
PROJECT DRAWINGS MUST BE PRINTED IN COLOUR FOR CLARITY.
NORMAL POWER
LIFE SAFETY
EXISTING



1 POWER & SERVICES - GROUND FLOOR - NEW WORK
E-301 1: 100



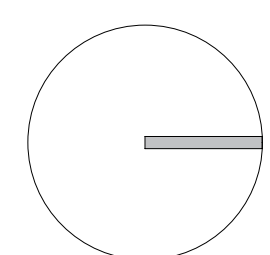
- GENERAL NOTES**
- A. UNLESS INDICATED OTHERWISE, ALL MATERIALS SHOWN ON THESE DRAWINGS TO BE NEW.
- B. MAINTAIN, RETAIN AND MAKE GOOD OF ALL EXISTING FEEDERS WHICH PASS THROUGH THE RENOVATION AREAS.
- C. THERE IS AN EXISTING TELCOR INTERCOM SYSTEM. ALL NEW DEVICES (SPEAKERS AND PHONES) SHALL BE COMPATIBLE WITH THE EXISTING SYSTEM TO ENSURE CONTINUITY, INTEGRITY AND FULL INTEROPERABILITY WITH THE FACILITY'S EXISTING TELCOR COMMUNICATION SYSTEM. ALL WORK SHALL BE PERFORMED BY THE DESIGNATED SYSTEM CONTRACTOR CAREFUL AT THE TIME OF TENDER. OBTAIN PROJECT SPECIFIC INSTALLATION INSTRUCTIONS, SCHEMATICS AND DIAGRAM FROM VENDORS. INCLUDE FOR ALL LABOR AND MATERIALS REQUIRED FOR THE COMPLETE AND FUNCTIONAL INSTALLATION OF THE PUBLIC ADDRESS SYSTEM.
- D. ALL STRUCTURAL DATA CABLE, TELECOMMUNICATIONS NETWORKS, NETWORKING HARDWARE TERMINATIONS, CLOCKS AND COMPLETE SECURITY SYSTEMS SHALL BE SUPPLIED, INSTALLED, COMPLETED AND COMMISSIONED EXCLUSIVELY BY THE DESIGNATED CONTRACTOR FC1. THE CONTRACTOR SHALL COORDINATE DIRECTLY WITH FC1 FOR ROUGH-IN TIMELINES, PATHWAYS, SCHEDULE PLACEMENTS, ENCLOSURE ROOM, SPEAKERS AND SYSTEMS. SCHEDULE PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. ALL DEVICE PROGRAMMING, SERVER INTERFACING, HEAD-END TERMINATIONS, TESTING, AND ULTIMATE CERTIFICATIONS SHALL BE CARRIED OUT BY FC1 CERTIFIED TECHNICIANS UPON SYSTEM COMPLETION.
- DRAWING NOTES**
1. RELOCATE POWER FEEDERS FOR EXISTING RTUs TO NEW DISTRIBUTION PANEL DP-02. EXISTING WIRING AND CONDUIT TO BE REMOVED, EXTENDED, AND SIZED AS NECESSARY TO MATCH NEW DISTRIBUTION. REMOVE AND REINSTALL THE ASSOCIATED INSULATED CASE CIRCUIT BREAKERS INTO DP-02. REFER TO THE SINGLE LINE DIAGRAM FOR MORE DETAILS.
2. NEW 500A, 12000V DISTRIBUTION PANEL DP-03 TO BE INSTALLED IN ELECTRICAL ROOM TO FEED NEW ELECTRICAL AND MECHANICAL LOADS ASSOCIATED WITH THE WING ADDITION. DP-03 TO BE FED BY TWO RUNS OF CABLE/CONDUIT FROM EXISTING DISTRIBUTION PANEL DP-01. REFER TO THE SINGLE LINE DIAGRAM FOR MORE DETAILS.
3. PROPOSED CONDUIT ROUTING FOR NEW PANEL. DETERMINE EXACT ROUTING ON SITE WITH OTHER SERVICES. REFER TO SINGLE LINE DIAGRAM ON DRAWING E-101 FOR WIRE AND CONDUIT SIZES.
4. PROVIDE JUNCTION BOX ABOVE ACCESSIBLE CEILING. COORDINATE EXACT LOCATION ON SITE WITH OTHER SERVICES.
5. NEW ELECTRONIC VALVE TRANSFORMER PROVIDED BY PLUMBING CONTRACTOR. ELECTRICAL TO PROVIDE 120V CONNECTIONS. COORDINATE CONNECTIONS WITH PLUMBER.
6. CABLE TRAY: GALVANIZED WIRE MESH, 305mm WIDE WITH DEPTH OF 50mm.
7. CONTRACTOR SHALL PROVIDE A 12U EIA-310 COMPLIANT WALL MOUNT IT CABINET MEASURING 22.575" X 22.575" X 17.75" WITH A LOCKABLE TEMPERED GLASS FRONT DOOR. THE ENCLOSURE MUST INCLUDE TWO PRE-INSTALLED ACTIVE COOLING FANS COMPATIBLE WITH THE INTEGRATED TOWER VENTILATION HOLES. PROVIDE AND MOUNT ONE HORIZONTAL DUCT TO PULL WITHIN THE CABINET TO SERVICE EQUIPMENT.
8. CORE THROUGH FOUNDATION WALL TO ROUTE CONDUITS BELOW GRADE INTO NEW SWITCHBOARD. REFER TO STRUCTURAL FOR DETAILS.
9. SAW CUT EXISTING SLAB AND EXCAVATE AS REQUIRED TO BRING IN NEW CONDUITS INTO SWITCHBOARDS. UPON COMPLETION, REPAIR/REPLACE EXISTING SLAB AND PROVIDE HOUSE KEEPING PAD. REFER TO STRUCTURAL FOR DETAILS.
10. NEW HOUSE KEEPING PAD. REFER TO STRUCTURAL FOR DETAILS.
11. CORE THROUGH EXISTING WALL TO ROUTE CONDUITS AS INDICATED. SCAN AND REVIEW CORE HOLE LOCATIONS WITH STRUCTURAL PRIOR TO WORK.



SEALS



PROJECT NORTH



CLIENT



ARCHITECT



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No.	DATE	ISSUED FOR	REVISION	A.W.	BY
1	2026-07-30	ISSUED FOR ADDENDUM ELE-001		A.W.	
0	2026-05-29	ISSUED FOR PERMIT AND TENDER		A.W.	

PROJECT
ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUT
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

DRAWING TITLE
ELECTRICAL
POWER & SERVICES
GROUND FLOOR
NEW WORK

DRAWING	DESIGN	VERIFICATION
N. LEMIEUX	J. CICHOCKI	A. WADDELL
BPA PROJECT No 25-1159	DRAWING No	
SCALE NTS		

E-301

No. ELE-002

Client:	Conseil des écoles catholiques du Centre-Est	
Project:	École Élémentaire Catholique J.-L. Couroux	
Project No.:	25-1159	
Description:	Revisions to drawings and clarifications to specifications	
Division(s):	26	
By:	Nicole Lemieux April Waddell, P. Eng	Date: 2026-08-06

This addendum forms an integral part of the original plans, specifications, and contractual documents. Bidders shall make sure that the cost of this addendum is included in the bid amount.

1. Documents

1.1 Included Documents

- Electrical Drawings:
 - E-000, revision 2.
 - E-100, revision 1.

2. Description of Work

2.1 Drawing E-000

- Refer to drawing for revised details 2 & 3.

2.2 Drawing E-100

- Revise feeder from hydro pole to switchboard SW-1 from 4#900 MCM + #2/0 GND to 2 x (4#600 MCM + #2 GND).
- Revise feeder from switchboard SW-1 to DP-1 from 2 x (4#350MCM-1C + GND TECK 90 CU XLPE 1000V) to 3 x (4#300MCM-1C + GND TECK 90 CU XLPE 1000V).

3. Clarifications

3.1 Question:

Q: Please confirm: Specification 26 05 34 2.3.4. are weatherproof fittings necessary? Please clarify?

A: The weatherproof specification for EMT fittings follows the National Master Specification (NMS) and is considered industry best practice. Because exact site conditions can fluctuate during or after installation, this specification ensures long-term compliance and protects from moisture ingress.

3.2 Question:

Q: *Section 26 24 16.01 2.3.1 indicates 240 V Panelboards: Bus and breakers rated for 35,000 A (symmetrical) interrupting capacity or as indicated. Drawing E-100 Single line shows no KAIC rating for the Switchboard, DP-1 and DP-2 but Drawing E-102 panel legend for New Panel BP-F shows 18 KA. Please advise on the right KA rating to use.*

A: For all panels where the interrupting capacity is not identified on the plans (SW-1, DP-1 and DP-2) the interrupting capacity is to be 35kA as per the specifications. For panel BP-F the interrupting capacity can be 18kA as indicated in the panel schedule.

For details, see attached documents.

MISCELLANEOUS

ELECTRICAL DISTRIBUTION

LIGHTING CONTROLS

SINGLE POLE SWITCH
"W" INDICATES CONTROLLED LIGHTING FIXTURE ZON

EMERGENCY LIGHTING & SIGNAGE

FIRE ALARM SYSTEM

PUBLIC ADDRESS

IDENTIFICATION

OUTLETS

STRUCTURED CABLING

GENERAL

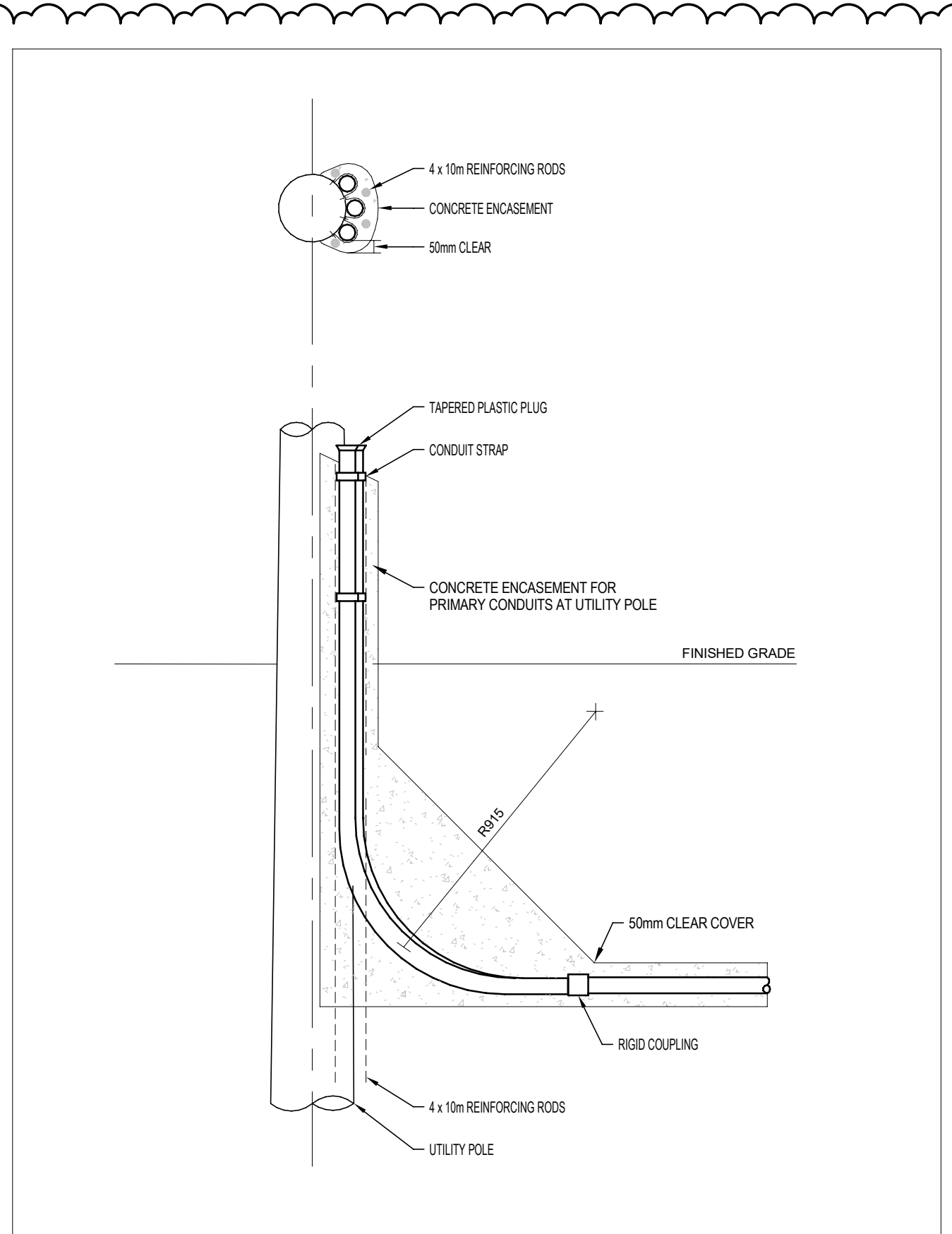
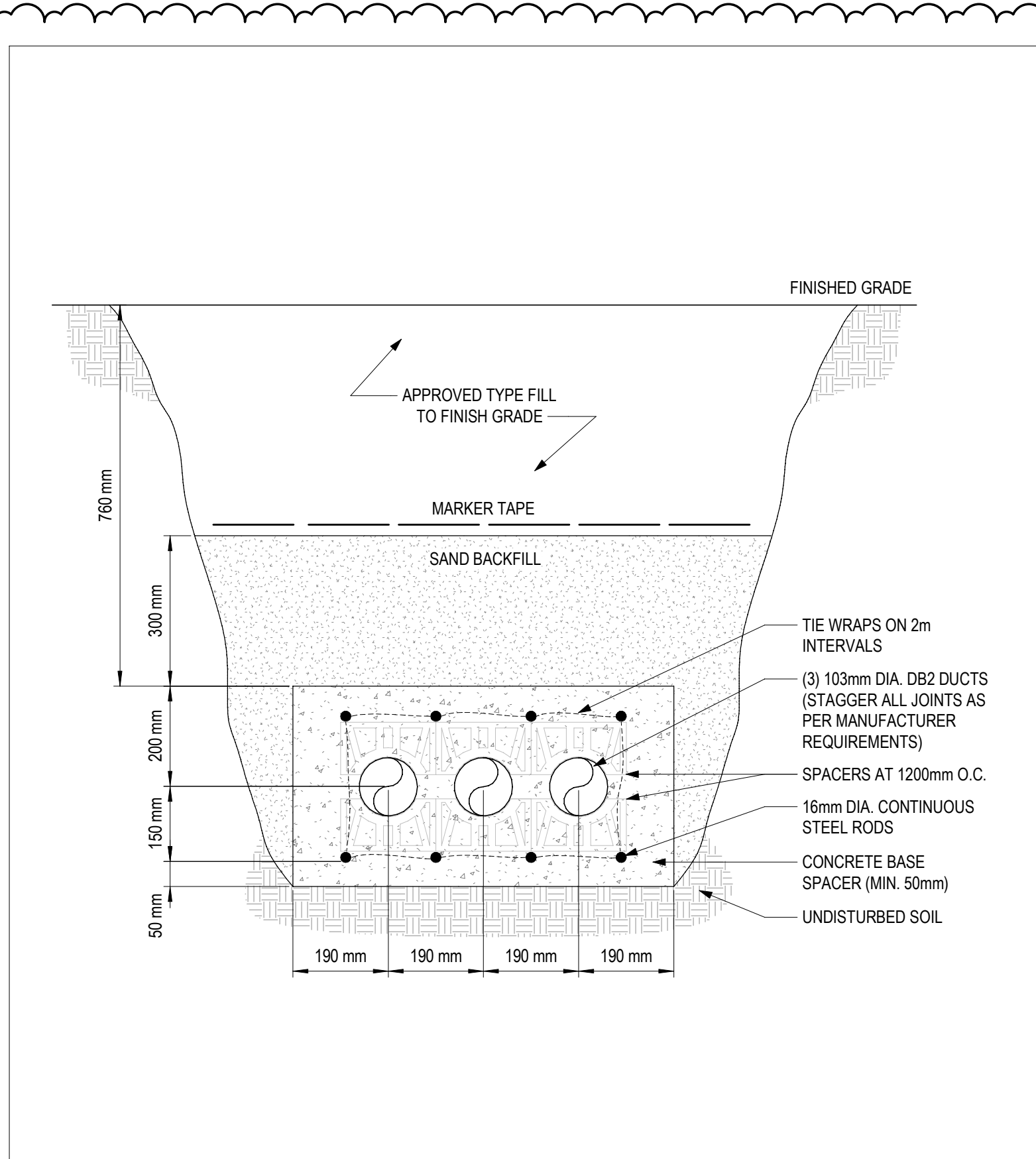
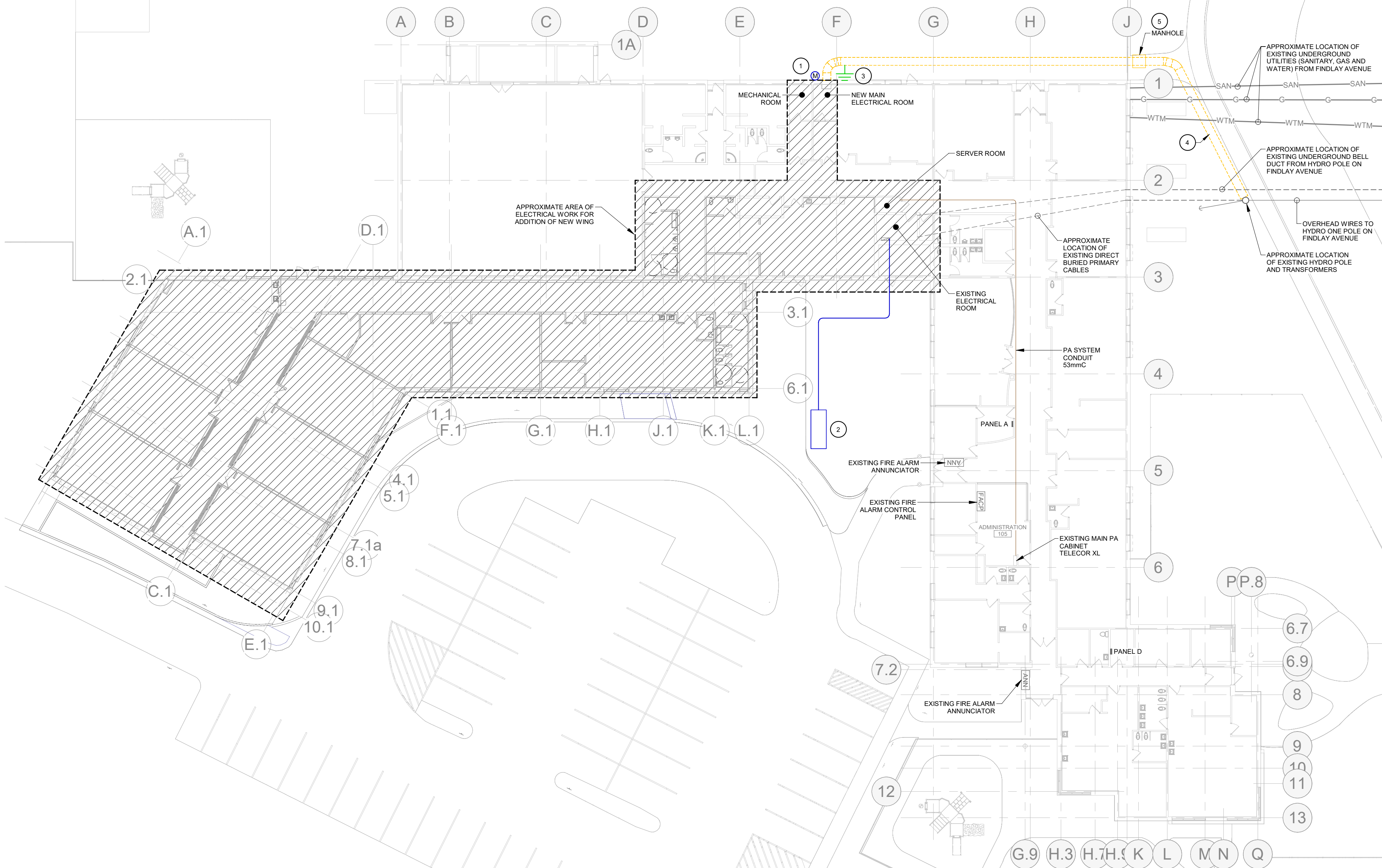
EQUIPMENT IDENTIFICATION

ELECTRICAL - KEY PLAN

CELL CONCRETE ENCASED DUCT BANK DETAIL

HYDRO TERMINATION AT POLE

PLAN LIST			
PLAN NO.	DISCIPLINE	SUB-DISCIPLINE	PLAN NAME
E-000	ELECTRICAL	GENERAL	LEGEND, DRAWING LIST & KEY PLAN
E-100	ELECTRICAL	GENERAL	DIAGRAMS
E-101	ELECTRICAL	GENERAL	LIGHTING SCHEDULES
E-102	ELECTRICAL	GENERAL	SCHEDULES
E-103	ELECTRICAL	GENERAL	DETAILS
E-104	ELECTRICAL	GENERAL	DETAILS
E-200	ELECTRICAL	LIGHTING	GROUND FLOOR - DEMOLITION
E-201	ELECTRICAL	LIGHTING	GROUND FLOOR - NEW WORK
E-300	ELECTRICAL	POWER & SERVICES	GROUND FLOOR - DEMOLITION
E-301	ELECTRICAL	POWER & SERVICES	GROUND FLOOR - NEW WORK
E-302	ELECTRICAL	POWER & SERVICES	NEW WORK
E-400	ELECTRICAL	FIRE ALARM	GROUND FLOOR - DEMOLITION
E-401	ELECTRICAL	FIRE ALARM	GROUND FLOOR - NEW WORK

[illegible]

DRAWING NOTES

1. ELECTRICAL CONTRACTOR TO RELOCATE EXISTING METERS AS INDICATED. PROVIDE THREE INCH EMT CONDUIT TO METERS (PT) AND CL-2 CONDUIT IN MAIN ELECTRICAL RACEWAY TO METER. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. WIRE BUNDLES BEFORE CONNECTION ARRANGE FOR PROTECTION OF CTS AND PDS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS.
2. DIVISION 26 TO PROVIDE 1500W 100% MODULATED TEMPORARY GENERATOR. DIESEL ENGINE AND GENERATOR FOR 24 HOURS OF SERVICE. DISBURSE 1/2" MINIMUM ALLOW 1/2" DRAIN OF FUEL AT 100% LOAD. COORDINATE ELECTRICAL SERVICE. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS.
3. PROVIDE AND INSTALL A COMPLETE GENERATOR. ELECTRODYNAMIC SYSTEM FOR MAIN ELECTRICAL RACEWAY. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS.
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5. PROVIDE AND INSTALL A COMPLETE GENERATOR. ELECTRODYNAMIC SYSTEM FOR MAIN ELECTRICAL RACEWAY. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS. PROVIDE 1/2" MIN. THICK 12" X 12" METAL PLATE TO PROTECT METERS.

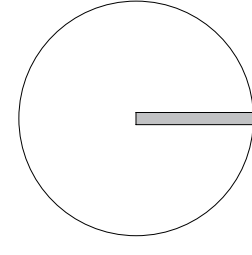
COLOURS LEGEND ELECTRICAL	
PROJECT DRAWINGS MUST BE PRINTED IN COLOUR FOR CLARITY.	
	NORMAL POWER
	GROUNDING/LIGHTNING PROTECTION
	DUCTBANK - ELECTRICAL
	EXISTING



SEALS



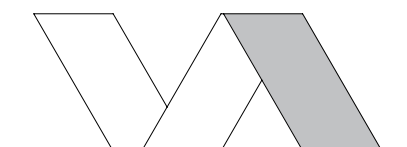
PROJECT NORTH



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tel. 613-797-0047 email. info@veerarch.com
340 Gladstone Ave., Suite 301, Ottawa ON K2P 0Y8

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2	2026-08-06	ISSUED FOR ADDENDUM ELE-002	A.W.
1	2026-07-30	ISSUED FOR ADDENDUM ELE-001	A.W.
0	2026-05-29	ISSUED FOR PERMIT AND TENDER	A.W.
No	DATE	REVISION	BY

PROJECT

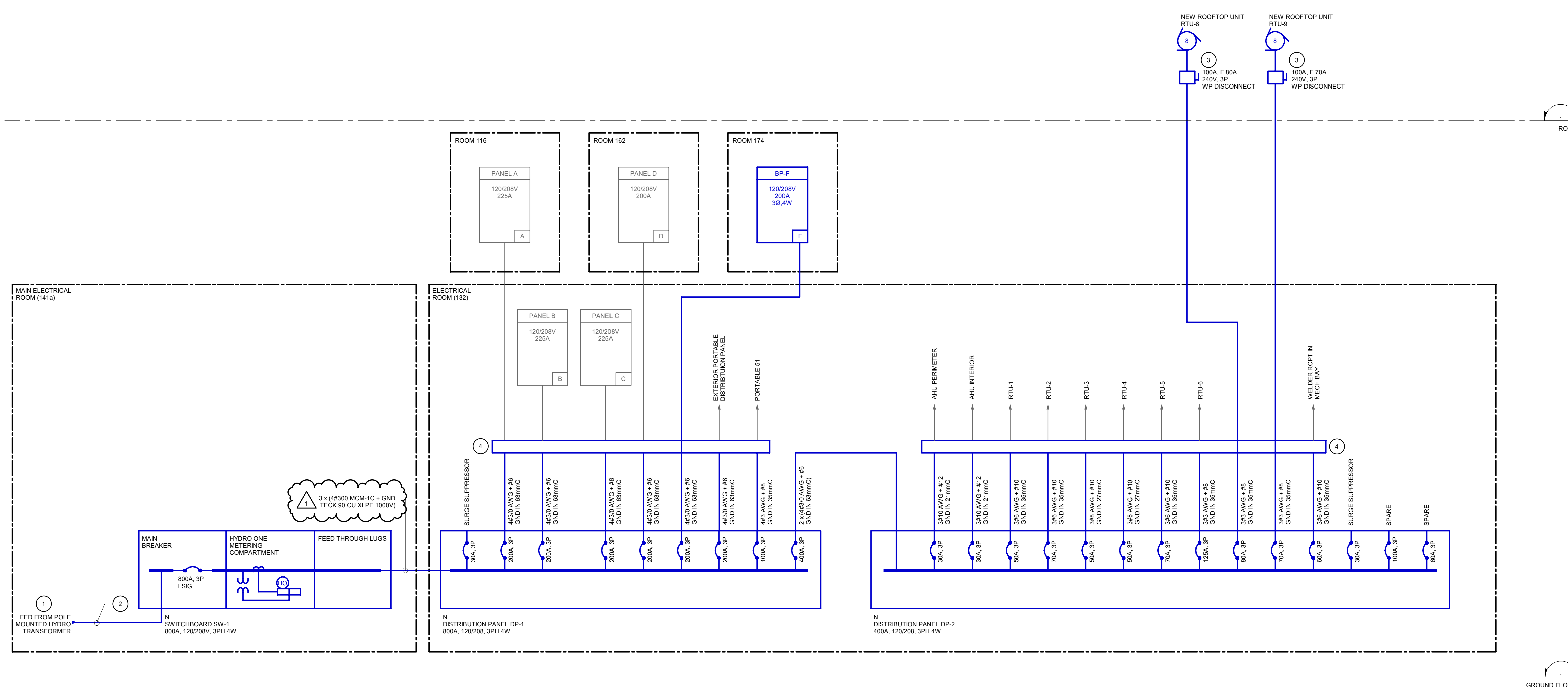
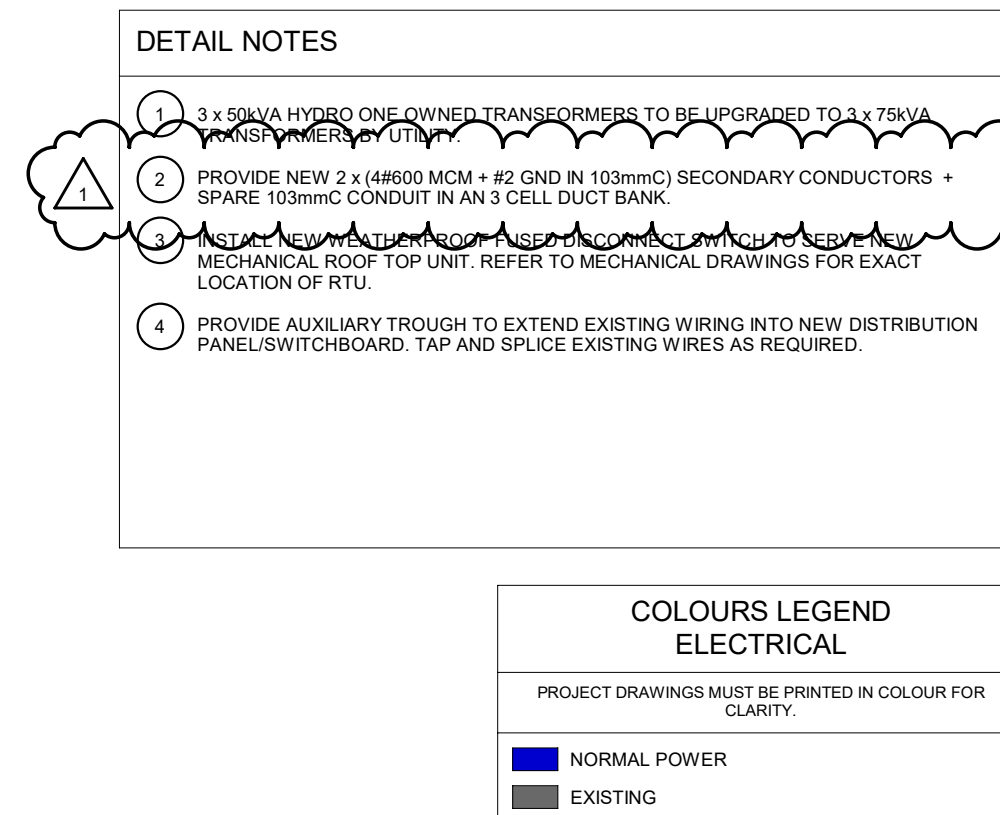
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CATHOLIQUE J.-L. COUROUT
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

DRAWING TITLE

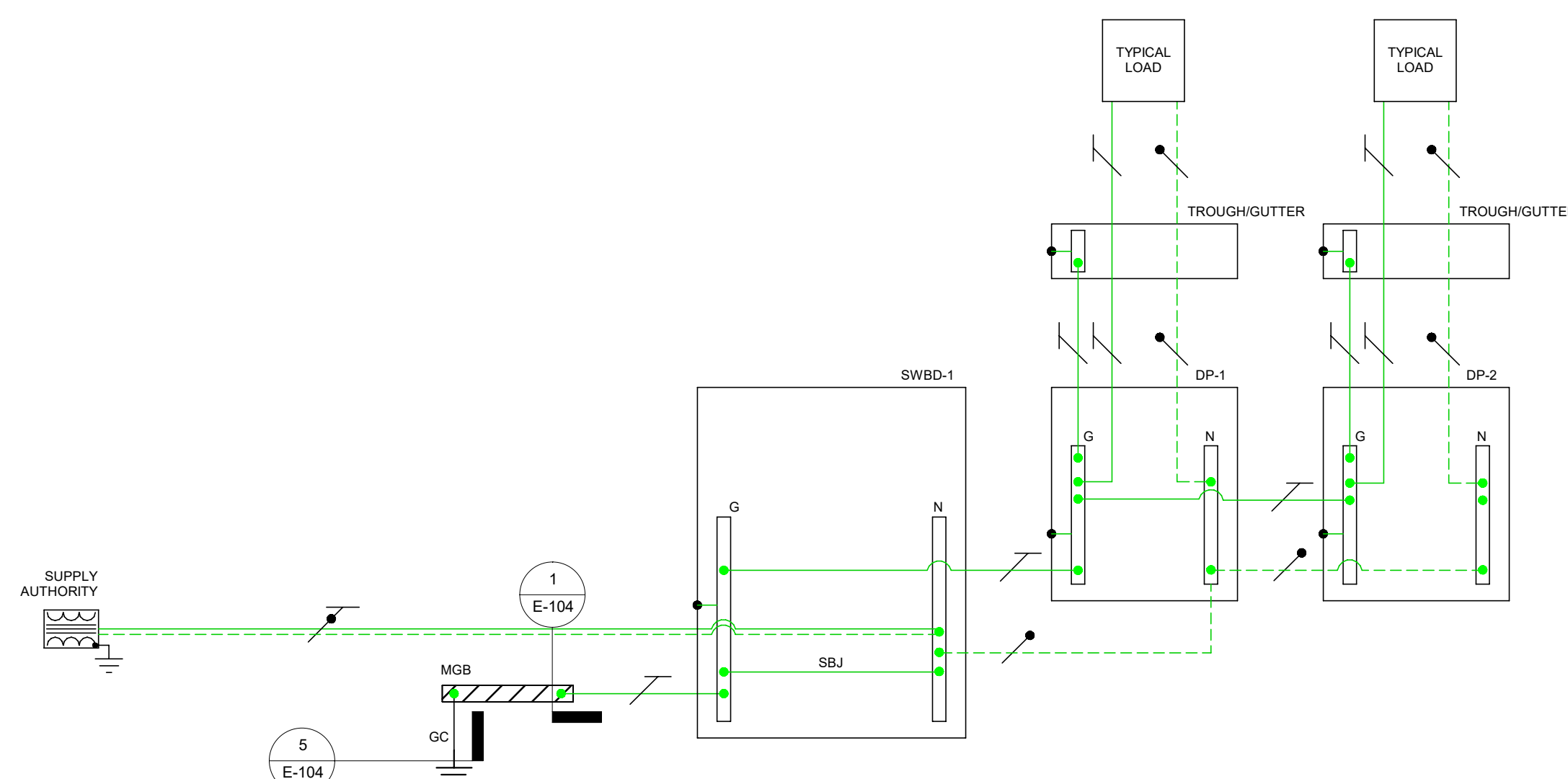
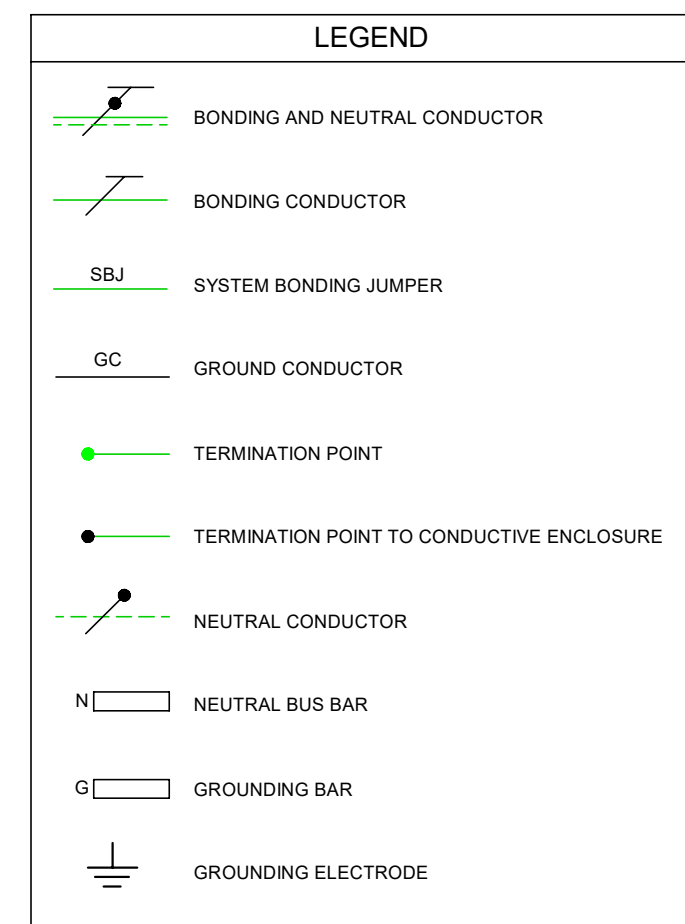
ELECTRICAL
GENERAL
LEGEND, DRAWING LIST & KEY
PLAN

DRAWING N. LEMIEUX	DESIGN J. CICHOCKI	VERIFICATION A. WADDELL
-----------------------	-----------------------	----------------------------

E-000



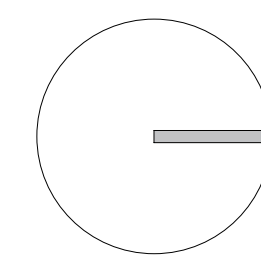
2 PARTIAL SINGLE LINE DIAGRAM - NEW WORK



4 GROUNDING AND BONDING DIAGRAM

SEALS

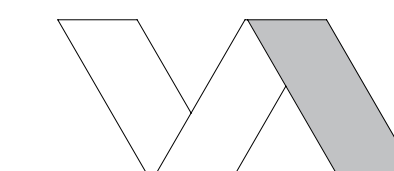
PROJECT NORTH



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ÉCOLE ÉLÉMENTAIRE
CATHOLIQUE J.-L. COUROUT
10 FINDLAY AVE
CARLETON PLACE
2025JLC117

DRAWING TITLE

ELECTRICAL
GENERAL
DIAGRAMS

DRAWING N. LEMIEUX	DESIGN J. CICHOCKI	VERIFICATION A. WADDELL
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BPA PROJECT

25-1159

SCA

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Geotechnical Investigation Proposed Addition to J.L. Couroux School 10 Findlay Avenue, Carleton Place

Client:

Conseil des Écoles Catholiques du Centre-Est
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Executive Summary

EXP Services Inc. (EXP) is pleased to present the results of the geotechnical investigation completed for the proposed addition to the J.L. Couroux catholic elementary School, located at 10 Findlay Avenue, Carleton Place, Ontario (Figure 1). Terms and conditions of this assignment were outlined in EXP Services Inc. (EXP) proposal number: OTT-25006852-A0 dated April 29, 2025. Authorization for EXP to proceed with this work was provided by Conseil des Écoles Catholiques du Centre-Est (CECCE) by signed service agreement for Project No. 2025JLC117 dated April 30, 2025.

The proposed addition will comprise of one storey with no basement to be constructed on the east side of the existing school building. Design details such as the finished floor elevation (FFE), underside of footing (USF) and exterior final grade (FG) elevations are not known at the time of this report. However, it has been assumed that the FFE of the proposed addition and final grades will match the existing.

The fieldwork for this geotechnical investigation was undertaken on June 5 and 6, 2025, and consists of the drilling of ten (10) boreholes (Borehole Nos. 25-1 to 25-10) to auger refusal and termination depths ranging from 0.6 m to 4.6 m below the existing grade (Elevation 137.2 m to 132.8 m). The borehole fieldwork was supervised on a full-time basis by EXP. The locations and geodetic elevations of the boreholes were established by EXP and are shown on the borehole location plan, Figure 2. Terms also call for the excavation of five (5) test pits as part of the scope of work. This work was differed until the final location of the proposed addition is known.

The borehole information indicates the subsurface conditions consist of a surficial topsoil or asphaltic concrete layer underlain by fill containing boulders and cobbles. The fill is underlain by sandy dolostone bedrock. Refusal to augers was met in all the boreholes at depths ranging between 0.4 m to 2.9 m below grade (Elevation 137.6 m to Elevation 134.5 m). Wash-boring and core drilling techniques used to advance below auger refusal depth in Borehole Nos. 25-06 and 25-08 indicated that refusal was met on dolostone bedrock with sandstone partings. The groundwater table at the site was established to range between 2.8 to 3.0 m depths (Elevation 135.0 m to Elevation 134.6 m). The groundwater is subject to seasonal fluctuations and may be at higher elevations during wet weather conditions.

Based on the currently available information, the Site Class for the proposed addition is "C" as per Table 4.1.8.4.-A and Table 4.1.8.4.-B, OBC 2024. According to Section A-4.1.8.4 of OBC 2024, the in-situ measurements of shear wave velocity can be utilized to lower the demand in the seismic design. Therefore, field shear wave velocity measurements are recommended to be performed through non-intrusive (e.g. multichannel analysis of surface waves) and / or intrusive (e.g. downhole / cross hole techniques, or SCPT) geophysical tests.

Compressible soils were not encountered at the borehole locations and therefore there is no grade raise restriction at the site from a geotechnical point of view.

The building addition may be supported by strip and spread footings founded on the sound sandy dolostone to sandstone bedrock, competent and free of soil filled seams and designed for a factored geotechnical resistance at Ultimate Limit State (ULS) of 1000 kPa. The factored ULS values includes a resistance factor of 0.5. The SLS bearing pressure of the bedrock, required to produce 25 mm settlement of the structure will be much larger than the recommended value for factored geotechnical resistance at ULS. Therefore, the factored geotechnical resistance at ULS will govern the design for footings founded on bedrock.

The floor slab for the proposed building may be designed and constructed as a slab-on-grade placed on a 200 mm thick 19 mm sized clear stone bed placed on a engineered fill pad set on the approved bedrock surface constructed in accordance with section [7](#) of this report which requires removal of the fill from below the slab and grade and its replacement with engineered fill. The clear stone would minimize the capillary rise of moisture from the sub-soil to the floor slab. Alternatively, the floor slab may be cast on a 200 mm thick bed of OPSS Granular A overlain by a vapour barrier. Adequate saw cuts should be provided in the floor slabs to control cracking.

Since there is no basement and the groundwater level was measured at 2.8 m to 3.0 m depth below ground (Elevation 134.6 m and Elevation 139.0 m), perimeter and underfloor drainage system will not be required for the proposed school building. However, If a perimeter drainage system is encountered along the existing building at the location of the proposed addition, then it should be reinstated around the new addition.

Excavations in overburden may be undertaken by conventional heavy mechanical equipment and should comply with the most recent Occupational Health and Safety Act (OHSA), Ontario Regulations 213/91. Excavation of side slopes in sound bedrock may be undertaken with near vertical sides. The most suited method to excavate any bedrock must be determined by contractor bidding on this project taking into consideration close proximity of the school building and type and strength of bedrock.

It is anticipated that excavations will be at or above the recorded groundwater level and seepage of the surface and subsurface water into the excavations is anticipated. However, it should be possible to remove any water entering the excavation by collecting water and pumping from the low points or sumps. In areas of high infiltration or in areas where more permeable soil layers may exist, a higher seepage rate should be anticipated and will require high-capacity pumps to keep the excavation dry.

It is anticipated that the majority of the material required for backfilling purposes in the interior and exterior of the proposed building, trench backfill, and pavements would have to be imported and should preferably conform to Ontario Provincial Standard Specification (OPSS) 1010.

Pavement structure for light duty traffic areas should consist of 50 mm thick asphaltic concrete, 150 mm thick OPSS Granular A base and 250 mm thick OPSS Granular B Type II subbase.

Test pit which forms part of EXP contract must be completed once the final location of the proposed addition is established and to determine if rough auguring was met on weathered bedrock or on boulders and cobbles likely present within the fill as shown on the logs of boreholes.

The above and other related considerations are discussed in greater detail in the main body of the attached geotechnical report.

1. Introduction

EXP Services Inc. (EXP) is pleased to present the results of the geotechnical investigation completed for the proposed addition to the J.L. Couroux catholic elementary School, located at 10 Findlay Avenue, Carleton Place, Ontario (Figure 1). Terms and conditions of this assignment were outlined in EXP Services Inc. (EXP) proposal number: OTT-25006852-A0 dated April 29, 2025, and authorization for EXP to proceed with this work was provided by Conseil des Écoles Catholiques du Centre-Est (CECCE) by signed service agreement for Project No. 2025JLC117 dated April 30, 2025.

The proposed addition will comprise of one storey with no basement proposed to be constructed on the east side of the existing school building. Design details such as the finished floor elevation (FFE), underside of footing (USF) and exterior final grade (FG) elevations are not known at the time of this report. However, it has been assumed that the FFE of the proposed addition and final grades will match the existing.

The geotechnical investigation was undertaken to:

- a) Establish the subsurface soil and groundwater conditions at ten (10) boreholes,
- b) Classify the site for seismic site response in accordance with the requirements of the 2024 Ontario Building Code and assess the potential for liquefaction of the subsurface soils during a seismic event,
- c) Comment on grade-raise restrictions and provide site grading requirements,
- d) Make recommendations regarding the most suitable type of foundations, founding depth and bearing pressure at serviceability limit state (SLS) and factored geotechnical resistance at ultimate limit state (ULS) of the founding strata and comment on the anticipated total and differential settlements of the recommended foundation type,
- e) Discuss Slab-on-grade construction and requirement for any underfloor or perimeter drainage system,
- f) Comment on excavation conditions and de-watering requirements during construction,
- g) Provide pipe bedding requirements for underground services;
- h) Discuss backfilling requirements and suitability of on-site soils for backfilling purposes,
- i) Recommend pavement structure thicknesses for access roads and parking lots, and
- j) Comment on the corrosion potential of subsurface soils to buried concrete and metal structures/members;

The comments and recommendations given in this report are based on the assumption that the above-described design concepts will proceed into construction. If changes are made either in the design phase or during construction, this office must be retained to review these modifications. The result of this review may be a modification of our recommendations, or it may require additional field or laboratory work to check whether the changes are acceptable from a geotechnical viewpoint.

2. Site Description

The J. L. Couroux Catholic Elementary School is located in the area bounded by Findlay Avenue to the North, King's Highway 7 to the South and Franktown Road to the east. Commercial properties exist to the west side boundary. The existing building consists of a one-storey structure with no basement.

The location of the proposed addition currently consists of landscaped grass covered ground with occasional trees and asphalt access roads and parking.

The topography of the addition location is relatively flat with borehole elevations ranging from Elevation 137.36 m to Elevation 138.35 m.

3. Procedure

The fieldwork for this geotechnical investigation was undertaken on June 5 and 6, 2025, and consists of the drilling of ten (10) boreholes (Borehole Nos. 25-1 to 25-10) to auger refusal and termination depths ranging from 0.6 m to 4.6 m below the existing grade (Elevation 137.2 m to 132.8 m). The borehole fieldwork was supervised on a full-time basis by EXP. The locations and geodetic elevations of the boreholes were established by EXP and are shown on the borehole location plan, Figure 2.

Prior to the fieldwork, the locations of the boreholes were cleared of any public and private underground services by a locator subcontracted to EXP. It is noted that the boreholes proposed to be drilled close to the existing building had been relocated due to the presence of a fiber optic line identified by the locator close to the building line. This service will require relocation for the construction of the proposed addition.

The boreholes were drilled using a CME-55LC track-mounted drill rig equipped with continuous flight hollow-stem auger equipment and rock coring capabilities, operated by a drilling contractor subcontracted to EXP. Standard penetration tests (SPTs) were performed in all the boreholes at the surface or under the asphalt layer and below the ground surface at 0.8 m depth intervals and the soil samples were retrieved by the split-spoon sampler. Two (2) boreholes were advanced beyond the depth of refusal by conventional coring techniques using the N-size core barrel. A field record of wash water return, colour of wash water and any sudden drops of the drill rods were kept during rock coring operations. The subsurface soil conditions in each borehole were logged with each soil sample, labelled and placed in plastic bags. Similarly, the rock cores were visually examined, placed in core boxes, identified, and logged.

Two (2) nineteen-millimeter diameter standpipes were installed, one (1) in each of Borehole Nos. 25-6 and 25-8 for long-term monitoring of the groundwater table. The standpipes were installed in accordance with EXP standard practice, and the installation configuration is documented on the respective borehole log. The boreholes were backfilled upon completion of drilling and installation of the standpipes.

On completion of the borehole fieldwork, the soil and rock samples were transported to the EXP laboratory in Ottawa where they were examined by a geotechnical engineer and logs of boreholes prepared. The soils are classified by their main constituents in accordance with the Unified Soil Classification System (USCS) using the soil group name and symbol and by the modified Burmister soil classification method for the classification of the minor constituents of the soil using adjectives and modifiers such as trace and some.

The geotechnical engineer also assigned the laboratory testing program which is summarized in Table I.

Table I: Summary of Laboratory Testing Program	
Type of Test	Number of Tests Completed
Soil Samples	
Moisture Content Determination	27
Grain Size Analysis	2
Rock Samples	
Unconfined Compressive Strength and Unit Weight Determination	4
Corrosion Analysis (pH, sulphate, chloride and resistivity)	1

4. Subsurface Conditions and Groundwater Levels

A detailed description of the subsurface conditions and groundwater levels from the boreholes are given on the attached Borehole Logs, Figures 3 to 12. The borehole logs and related information depict subsurface conditions only at the specific locations and at the times indicated. Subsurface conditions and water levels at other locations may differ from conditions at the locations where sampling was conducted. The passage of time also may result in changes in the conditions interpreted to exist at the locations where sampling was conducted.

It should be noted that the soil boundaries indicated on the borehole logs are inferred observations during drilling operations. These boundaries are intended to reflect approximate transition zones for the purpose of geotechnical design and should not be interpreted as exact planes of geological change. The “Notes on Sample Descriptions” preceding the borehole logs form an integral part of this report and should be read in conjunction with this report.

A review of the borehole logs indicates the following subsurface conditions with depth and groundwater level measurements.

4.1 Topsoil

An 85 mm to 200 mm thick surficial topsoil layer was encountered in all the boreholes except Borehole No. 25-7.

4.2 Asphalt

A 40 mm thick asphalt layer was encountered at Borehole No. 25-7.

4.3 Granular Fill

Underlying the asphalt layer at Borehole No. 25-7 is crushed limestone granular fill with a thickness of 110 mm.

4.4 Fill

The surficial topsoil or pavement layer is underlain by fill in all the boreholes and extends to 0.4 m to 2.9 m depths (Elevation 137.6 m to Elevation 134.5 m). The fill ranges in consistency from silty clay to silty sand with gravel, organics such as roots and topsoil, and possible debris, cobbles or boulders. The fill in a very loose to dense state based on standard penetration tests (SPT) N-values ranging from 2 to 43. The moisture content ranged from 3 to 20 percent. The deep fill in Borehole No.25-06 is likely due to previous excavation completed in the general area of the site.

Results from grain-size analysis conducted on two (2) samples of the fill are summarized in Table II. The grain-size distribution curves are shown in Figures 13 and 14.

Table II: Summary of Results from Grain-Size Analysis – Fill Samples						
Borehole No. (BH) – Sample No. (SS)	Depth (m)	Grain-Size Analysis (%)				Soil Classification
		Gravel	Sand	Silt	Clay	
BH25-6 - SS2	0.8 - 1.4	37	36	27	0	Silty Sand and Gravel (SM / GM)
BH25-10 - SS1	0.0 - 0.6	33	52	15	0	Silty Sand (SM), Gravelly

Based on a review of the results of the grain-size analysis, the soil may be classified as gravelly silty sand (SM) to silty sand and gravel (SM / GM).

4.5 Auger Refusal and Bedrock

Auger refusal was met at 0.4 m to 2.9 m depths (Elevation 137.6 m to Elevation 134.5 m) in all the boreholes.

A summary of the auger refusal depths as well as the depth of bedrock confirmed by coring are shown in Table III.

Table III: Summary of Auger Refusal and Bedrock Depths (Elevations) in Boreholes				
Borehole (BH) No.	Ground Surface Elevation (m)	Refusal Depth (m) (Elevation (m))	Depth (Elevation) of Proven Bedrock (m)	Comment wrt to Depth (Elevation) of Bedrock Surface
BH25-01	137.85	1.0 (136.9)	--	Auger refusal at 1.0 m
BH25-02	137.53	1.5 (136.0)	--	Auger refusal at 1.5 m
BH25-03	138.25	1.1 (137.2)	--	Auger refusal at 1.1 m
BH25-04	137.36	0.6 (136.8)	--	Auger refusal at 0.6 m
BH25-05	137.73	1.0 (136.7)	--	Auger refusal at 1.0 m
BH25-06	137.42	2.9 (134.5)	2.9 (134.5)	1.8 m length of bedrock cored below 2.9 m depth
BH25-07	137.95	1.5 (136.5)	--	Auger refusal at 1.5 m
BH25-08	137.97	0.4 (137.6)	0.4 (137.6)	2.7 m length of bedrock cored below 0.4 m depth
BH25-09	137.56	0.8 (136.8)	--	Auger refusal at 0.8 m
BH25-10	138.35	1.2 (137.2)	--	Auger refusal at 1.2 m

A review borehole logs indicates that the total core recovery (TCR) ranges between 85 percent and 100 percent and the rock quality designation (RQD) ranges between 51 and 100 indicating the bedrock is of fair to excellent quality. Photographs of the bedrock cores are included in Appendix A.

The bedrock geology map Paleozoic Geology of the Carleton Place Area, Southern Ontario; Ontario Geological Survey Map P.2725 Geological Series – Preliminary Map, scale 1:50000. Geology 1982, indicates that the site is underlain by March Formation Interbedded quartz sandstone, sandy dolostone, and dolostone.

Unit weight determination and unconfined compressive strength tests were conducted on four (4) rock core samples. The test results are summarized in Table IV.

Table IV: Summary of Unconfined Compressive Strength Test Results – Bedrock Cores				
Borehole (BH) No. – Run No.	Depth (m)	Unit Weight (kN/m ³)	Unconfined Compressive Strength (MPa)	Classification of Rock with respect to Strength
BH25-6 Run2 (Top)	3.1 - 3.3	27.6	228	Very Strong R5
BH25-6 Run2 (Bottom)	4.2 - 4.4	27.5	207	Very Strong R5
BH25-8 Run1	0.9 – 1.0	27.1	163	Very Strong R5
BH25-8 Run 2	1.9 – 2.1	27.8	159	Very Strong R5

A review of the test results in Table IV indicates the strength of the rock may be classified as **very strong** (R5) in accordance with the Canadian Foundation Engineering Manual (CFEM), Fifth Edition, 2023.

4.6 Groundwater Level Measurements

Groundwater levels measured within the standpipes installed in Borehole Nos. 25-06 and 25-08 on June 16, 2025 indicated the water to be in the bedrock at depths of 2.8 m to 3.0 (Elevation 135.0 m to 134.6 m).

The groundwater levels were determined in the boreholes at the time and under the condition stated in this report. Note that fluctuations in the level of groundwater may occur due to a seasonal variation such as precipitation, snowmelt, rainfall activities, and other factors not evident at the time of measurement and therefore may be at a higher level during wet weather periods.

5. Site Classification for Seismic Site Response and Liquefaction Potential of Soils

The recommendations for the geotechnical aspects to determine the earthquake loading for design in accordance with Section 4.1.8 Earthquake Load and Effects in the Ontario Building Code (OBC) 2024, are presented below.

5.1 Subsoil Conditions

The strata and groundwater information at this site have been examined in relation to Section 4.1.8.4 of the OBC (2024). The strata generally consist of fill and sandy dolostone bedrock. It is anticipated that the proposed basementless addition will be supported on conventional spread and strip footings founded on the bedrock surface.

5.2 Borehole Depth and In Situ Measurements

Table 4.1.8.4.-A Exceptions for Site Designation Using V_{s30} Calculated from In Situ Measurements and Table 4.1.8.4.-B Site Classes, S , for Site Designation X_s in OBC (2024) indicated that to determine the site classification, the average properties in the top 30 m (below the lowest basement level) are to be used. Site Classification can be determined using the average shear wave velocity (V_{s30}) as per the classifications stated in Table 4.1.8.4.-A and Table 4.1.8.4.-B. If in-situ shear wave velocity measurements are not available, the site designation X_s shall be determined based on the energy-corrected average standard penetration resistance (SPT) $\bar{N}_{60}$ or the average undrained shear strength S_u in accordance with Table 4.1.8.4.-B.

There are no shear wave measurements carried out at this site and therefore, the Site Designation will be determined based on the energy-corrected average SPT. The boreholes advanced at this site terminated at depths of 0.6 m to 4.6 m below existing grade. Therefore, the recommended site classification would be based on the available information as well as our interpretation of conditions below the boreholes based on our knowledge of the soil conditions in the subject area.

5.3 Site Classification and Liquefaction Potential of Soils

Based on the above assumptions and currently available information, the Site Class for the proposed structure is "C" as per Table 4.1.8.4.-A and Table 4.1.8.4.-B, OBC 2024. According to Section A-4.1.8.4 of OBC 2024, the in-situ measurements of shear wave velocity can be utilized to lower the demand in the seismic design. Therefore, field shear wave velocity measurements are recommended to be performed through non-intrusive (e.g. multichannel analysis of surface waves) and / or intrusive (e.g. downhole / cross hole techniques, or SCPT) geophysical tests.

The subsurface soils are not considered to be liquefiable during a seismic event.

6. Grade Raise Restrictions

Compressible soils were not encountered at the borehole locations and therefore there is no grade raise restriction at the site from a geotechnical point of view.

7. Site Grading

Site grading within the footprint of the **proposed addition** for the construction of the slab on grade should consist of the excavation and removal of the surficial topsoil layer and existing fill soils down to the bedrock surface. The exposed bedrock should be examined by a geotechnician. Once the bedrock subgrade has been approved, the grades may be raised to within 200 mm of the design underside of floor slab elevation by the construction of an engineered fill pad comprising of OPSS Granular B Type II material placed in 300 mm thick lifts and each lift compacted to 98 percent SPMDD. The placement and compaction of the engineered fill can in this way be undertaken to within 200 mm of the underside of the concrete floor slab with the remaining 200 mm backfilled using clear stone or OPSS Granular A compacted to 100 percent SPMDD and a vapor barrier. The engineered fill should be placed under the full-time supervision of a geotechnician working under the direction of a geotechnical engineer. In-place density tests should be undertaken on each lift of the engineered fill to ensure that it is properly compacted prior to placement of a subsequent lift.

Site grading within the **parking area** should consist of the removal of the surficial topsoil layer and organic stained soils and the exposed soil proof-rolled with a heavy vibratory roller in the presence of a geotechnician. Any loose/soft areas identified during the proof-rolling process should be excavated, removed and replaced with Ontario Provincial Standard Specification (OPSS) Granular B Type II or OPSS Select Subgrade Material (SSM) compacted to 95 percent standard Proctor maximum dry density (SPMDD).

8. Foundation Considerations

It has been assumed that the design FFE of the addition will be set at approximately the same elevation as the existing building. It has been assumed that the existing footings are founded on the bedrock surface.

A review of logs for the boreholes located within the proposed building addition footprint indicates that the depth of auger refusal was met at 0.4 m to 2.9 m depths (Elevation 137.6 m to Elevation 134.5 m). The presence of very strong sandy dolostone with minor sandstone bedrock was confirmed in Borehole Nos. BH25-06 and BH25-08 at depths of 2.9 m and 0.4 (Elevation 134.5 m and Elevation 137.6 m), respectively. It is suspected that the deep fill of 2.9 m contacted in Borehole No. 25-06 is likely due to previous excavation, i.e. trench in this area.

The building addition may be supported by strip and spread footings founded on the sound sandy dolostone to sandstone bedrock, competent and free of soil filled seams. The footings may be designed for a factored geotechnical resistance at Ultimate Limit State (ULS) of 1000 kPa. The factored ULS values includes a resistance factor of 0.5. The SLS bearing pressure of the bedrock, required to produce 25 mm settlement of the structure will be much larger than the recommended value for factored geotechnical resistance at ULS. Therefore, the factored geotechnical resistance at ULS will govern the design for footings founded on bedrock. In the area of Borehole No. 25-06, fill is required to be removed and replaced with lean mix concrete (15MPa) to the underside of the footing. The extent of the area of deep fill is not known.

All footing beds should be examined by a geotechnical engineer/technician to ensure that the founding surfaces are capable of supporting the design bearing pressure at ULS and that the footing beds have been properly prepared.

It is recommended that test pits are carried out to verify the depth and founding conditions of the existing footings on the east side of the existing building. For the construction of new footings immediately adjacent to the existing footings, the new footings should be ideally founded at the same level as the existing footings to eliminate the need for underpinning.

For footings of a heated structure where all the footings are founded on the sound bedrock, it is recommended the soil cover be 1.2 m to protect the footings from frost damage. For footings of an unheated structure founded on sound bedrock, the soil cover should be 1.5 m. Alternatively, frost protection may be provided by rigid insulation or by a combination of rigid insulation and earth cover.

The recommended factored geotechnical resistance at ULS and bearing pressure at SLS have been calculated by EXP from the borehole information for the design stage only. The investigation and comments are necessarily on-going as new information of underground conditions becomes available. For example, more specific information is available with respect to conditions between boreholes when foundation construction is underway. The interpretation between boreholes and the recommendations of this report must therefore be checked through field monitoring provided by an experienced geotechnical engineer to validate the information for use during the construction stage.

9. Floor Slab and Drainage Requirements

The floor slab for the proposed building may be designed and constructed as a slab-on-grade placed on a 200 mm thick 19 mm sized clear stone bed placed on a minimum 300 mm thick engineered fill pad set on the approved bedrock surface constructed in accordance with section [7](#) of this report. The clear stone would minimize the capillary rise of moisture from the sub-soil to the floor slab. Alternatively, the floor slab may be cast on a 200 mm thick bed of OPSS Granular A overlain by a vapour barrier. Adequate saw cuts should be provided in the floor slabs to control cracking.

Since there is no basement and the groundwater level was measured at 2.8 m to 3.0 m depth below ground (Elevation 134.6 m and Elevation 139.0 m), a perimeter and underfloor drainage system will not be required for the proposed school building. If a perimeter drainage system is encountered along the existing building at the location of the proposed addition, then it should be reinstated around the new addition.

The floor slab should be set at a minimum of 150 mm higher than the surrounding final exterior grade.

The final exterior grade surrounding the proposed building should be sloped away from the proposed building to prevent ponding of surface water close to the exterior walls of the proposed building.

10. Excavation and De-Watering Requirements

10.1 Excess Soil Management

EXP completed excess soils sampling as part of this investigation and the findings are presented under a different cover.

10.2 Excavations

This section of the report discusses excavation requirements for the construction of the building addition, the construction of the underground services and the new surface parking lot.

Excavation for the construction of footings and the installation of underground services anticipated to extend to a maximum depth of 2.9 m below the existing grade approximately at Elevation 134.5 m and will extend through the existing asphaltic concrete, topsoil and fill, and into the sandy dolostone bedrock. The groundwater level ranges from 2.8 to 3.0 m depths (Elevation 134.6 m to Elevation 135.0 m). It is anticipated that the excavation will be at or above the recorded groundwater level.

10.2.1 Overburden (Soil) Excavation

Excavations in overburden soil may be undertaken by conventional heavy mechanical equipment capable of removing cobbles, boulders, or possible large debris within the fill.

The excavation within the subsurface soils should comply with the most recent Occupational Health and Safety Act (OHSA), Ontario Regulations 213/91. Based on the definitions contained in OHSA, the subsurface soils above the water table at the site are classified as Type 3 soil and sidewalls of open cut excavations must be cut back at 1H:1V from the bottom of the excavation. Below the groundwater table, the excavation side slopes are expected to slough and will eventually stabilize at a slope of 2H:1V to 3H:1V. If the required side slopes for open cut excavations cannot be achieved due to space restrictions on site, such as the proximity of the open cut excavation to property limits, properly dewatered excavations for the installation of underground services may be undertaken within the confines of a prefabricated support system (trench box) designed and installed in accordance with OHSA.

Many geologic materials deteriorate rapidly upon exposure to meteorological elements. Unless otherwise specifically indicated in this report, walls and floors of excavations must be protected from moisture, desiccation, and frost action throughout the course of construction.

10.2.2 Bedrock Excavations

It is anticipated that excavations will extend into the sandy dolostone bedrock for footings and underground services installation. The excavation of side slopes in the sound sandy dolostone bedrock may be undertaken with near vertical sides subject to examination by a geotechnical engineer.

Contractors bidding on this project should decide on their own the most preferred rock removal method, hoe ramming or line drilling and blasting.

10.2.3 Preconstruction Survey and Vibration Monitoring

It is recommended that a pre-construction condition survey of the existing school and surrounding infrastructure located within the construction zone of influence be undertaken prior to any earth (soil) and rock excavation work as well as vibration monitoring during excavation, blasting and construction operations. Prior to the commencement of blasting, a detailed blast methodology should be submitted by the Contractor.

10.3 De-Watering Requirements

It is anticipated that excavations will be at or above the recorded groundwater level and seepage of the surface and subsurface water into the excavations is anticipated. However, it should be possible to remove any water entering the excavation by

collecting water and pumping from the low points or sumps. In areas of high infiltration or in areas where more permeable soil layers may exist, a higher seepage rate should be anticipated and will require high-capacity pumps to keep the excavation dry.

For construction dewatering, an Environmental Activity and Sector Registry (EASR) approval may be obtained for water takings greater than 50 m³ and less than 400 m³ per day. If more than 400 m³ per day of groundwater are generated for dewatering purposes, then a Category 3 Permit to Take Water (PTTW) must be obtained from the Ministry of the Environment, Conservation and Parks (MECP). A Category 3 PTTW would require a complete hydrogeological assessment and would take at least 90 days for the MECP to process once the application is submitted.

Although this investigation has estimated the groundwater levels at the time of the fieldwork, and commented on dewatering and general construction problems, conditions may be present which are difficult to establish from standard boring and excavating techniques and which may affect the type and nature of dewatering procedures used by the contractor in practice. These conditions include local and seasonal fluctuations in the groundwater table, erratic changes in the soil profile, thin layers of soil with large or small permeabilities compared with the soil mass, etc. Only carefully controlled tests using pumped wells and observation wells will yield the quantitative data on groundwater volumes and pressures that are necessary to adequately engineer construction dewatering systems.

11. Backfilling Requirements and Suitability of On-Site Soils for Backfilling Purposes

The on-site soils to be excavated are topsoil, fill, and shattered sandy dolostone rock. Portions of the on-site fill may be re-used as select subgrade material in the exterior of the building pending additional testing during construction. Otherwise, the fill may be used for landscaping purposes provided it is free of organics, cobbles, boulders and debris. Excavated bedrock is not suitable for use as backfill and should be discarded.

It is anticipated that the majority of the material required for backfilling purposes in the interior and exterior of the proposed building, trench backfill and parking lot would have to be imported and should preferably conform to the following specifications:

- Granular subbase for pavements, and underfloor fill including backfilling in service trenches inside the proposed addition - OPSS 1010 Granular B Type II placed in 300 mm thick lifts and each lift compacted to 100 percent SPMDD,
- Granular base for pavements, and underfloor fill within the top 200 mm of the slab - OPSS 1010 Granular A placed in 200 mm thick lifts and each lift compacted to 100 percent SPMDD,
- Backfill material against foundation walls located outside the proposed school addition – OPSS 1010 Granular B Type II placed in 300 mm thick lifts and each lift compacted to 95 percent SPMDD,
- Trench backfill outside building area, and fill placement to subgrade level for pavement – Approved on-site material or imported OPSS 1010 Select Subgrade Material (SSM), free of organics, debris and with a natural moisture content within 2 percent of the optimum moisture content. It should be placed in 300 mm thick lifts compacted to minimum 95 percent SPMDD; and
- Fill for landscaped areas should be clean fill free of debris, topsoil (organic soil), cobbles and boulders placed in 300 mm thick lifts and each lift compacted to 92 percent SPMDD.

12. Pipe Bedding Requirements

It is anticipated that the subgrade for the proposed underground services will be in the dolostone bedrock.

The pipe backfill including material specifications, thickness of cover material and compaction requirements should conform to municipal requirements and/or Ontario Provincial Standard Specification and Drawings (OPSS and OPSD). The pipe bedding should consist of 150 mm thick of OPSS Granular A bedding material. The bedding material should be compacted to at least 98 percent SPMDD.

For paved surfaces that will be located over service trenches, it is recommended that the trench backfill material within the 1.8 m frost zone, should match the existing material exposed along the trench walls to minimize differential frost heaving of the subgrade. The trench backfill should be placed in 300 mm thick lifts and each lift should be compacted to 95 percent SPMDD. Alternatively, frost tapers may be used.

To minimize the potential for bending stresses within the pipe, a transition zone treatment should be provided in areas where the pipe subgrade changes from overburden to bedrock and vice versa. In areas where the surface of the bedrock slopes at a steeper gradient than 3H:1V, the bedrock should be excavated and additional bedding material placed to create a 3H:1V transition zone.

13. Pavement Structure for Parking Lots

The subgrade for the pavement structures is anticipated to consist of the existing fill and sandy dolostone bedrock. Pavement structure thicknesses required for the parking lots set on the anticipated approved subgrade materials were computed and are shown in Table V. The pavement structures assume a functional design life of 15 to 20 years. The proposed functional design life represents the number of years to the first rehabilitation, assuming regular maintenance is carried out.

Table V: Recommended Pavement Structure Thicknesses

Pavement Layer	Compaction Requirements	Light Duty Traffic (Cars Only)
Asphaltic Concrete (PG 58-34)	92 percent to 97 percent MRD	65 mm HL3/SP12.5 mm Cat. B
OPSS 1010 Granular A Base	100% percent SPMDD	150 mm
OPSS 1010 Granular B Type II Sub-base	100% percent SPMDD	300 mm
Notes: 1. SPMDD denotes standard Proctor maximum dry density, ASTM, D-698-12e2. 2. MRD denotes Maximum Relative Density, ASTM D2041. 3. The upper 300 mm of the subgrade fill must be compacted to 98% SPMDD.		

The foregoing design assumes that construction is carried out during dry periods and that the subgrade is stable under the load of construction equipment. If construction is carried out during wet weather and, heaving or rolling of the subgrade is experienced, additional thickness of granular material may be required in addition to geotextile fabric.

Additional comments on the construction of the parking lots are as follows:

1. As part of the subgrade preparation, the areas of the proposed parking area should be stripped of all existing fill, surficial and buried topsoil (organic) layers and organic stained soils down to the subgrade level. The subgrade should be properly shaped, crowned, then proof-rolled in the full-time presence of a representative of this office. Any soft or spongy subgrade areas detected should be sub excavated and properly replaced with suitable approved backfill compacted to 95 percent SPMDD (ASTM D698-12e2).
2. The finished pavement surface should be free of depressions and should be sloped (preferably at a minimum cross fall of 2 percent) to provide effective surface drainage towards catch basins. Surface water should not be allowed to pond adjacent to the outside edges of paved areas.
3. The granular materials used for pavement construction should conform to Ontario Provincial Standard Specifications (OPSS 1010) for Granular A and Granular B Type II and should be compacted to 100 percent of the SPMDD.

The asphaltic concrete used, and its placement should meet OPSS 1150 or 1151 requirements. It should be compacted from 92 percent to 97 percent of the MRD (ASTM D2041). Asphalt placement should be in accordance with OPSS 310 and OPSS 313.

It is recommended that EXP be retained to review the final pavement structure design and drainage plans prior to construction to ensure they are consistent with the recommendations of this report.

14. Subsurface Concrete Requirements and Corrosion Potential

Chemical tests limited to pH, sulphate, chloride and resistivity were collected on a selected section of the bedrock cores and the results are presented in Table VI. The laboratory certificate of analysis is provided in Appendix B.

Table VI: Results of pH, Sulphate, Chloride and Resistivity Tests on Rock Samples						
Borehole (BH) No. – Run No.	Depth (m)	Material	pH	Chloride Content (%)	Sulphate Content (%)	Resistivity (Ohm*cm)
BH25-8 - Run1	0.5 - 0.6	Sandy Dolostone	9.71	0.004	0.0013	6210

The results indicate that the sandy dolostone bedrock has sulphate and chloride content of less than 0.002 percent and 0.004 percent respectively. These concentrations of sulphate and chloride would have a negligible potential of sulphate and chloride attack on subsurface concrete. The concrete should be in accordance with Table Nos. 3 and 6 of CSA A.23.1-14. However, the concrete should be dense, well compacted and cured.

The results of the resistivity test indicates that the sandy dolostone bedrock is mildly corrosive to bare steel. Care should be taken to protect buried steel elements from corrosion.

15. Additional Comments

It is recommended that test pits are carried out at the existing foundation adjacent to the proposed addition to verify the depth and founding conditions of the existing footings and assist in the design of the proposed building addition foundation.

The test pits which form part of the EXP contract must be completed once the final location of the proposed addition is established and to determine if rough auguring was met on weathered bedrock or on boulders and cobbles likely present within the fill as shown on the logs of boreholes.

Based on the additional test hole information, the geotechnical engineering comments and recommendations indicated in this report may need to be updated.

All earthwork activities from subgrade preparation to placement and compaction of engineered fill, fill in service trenches, placement and compaction of granular materials and asphaltic concrete, should be inspected by a qualified geotechnician to ensure that construction proceeds according to the project specifications.

All the footing beds should be examined by a geotechnical engineer to ensure that the founding surfaces are capable of supporting the design bearing pressure and that the footing beds have been properly prepared.

16. General Comments

The comments given in this report are intended only for the guidance of design engineers. The number of boreholes required to determine the localized underground conditions between boreholes affecting construction costs, techniques, sequencing, equipment, scheduling, etc., would be much greater than has been carried out for the design purposes. Contractors bidding on or undertaking the works should, in this light, decide on their own investigations, as well as their own interpretations of the factual borehole results, so that they may draw their own conclusions as to how the subsurface conditions may affect them.

The information contained in this report is not intended to reflect on environmental aspects of the soils. Excess soil sampling was completed as part of this work and findings are provided under a separate cover.

We trust that the information contained in this report will be satisfactory for your purposes. Should you have any questions, please do not hesitate to contact this office.

Sincerely,

EXP Services Inc.



Matthew Zammit, M.A.Sc., P.Eng.
Geotechnical Engineer
Earth & Environment

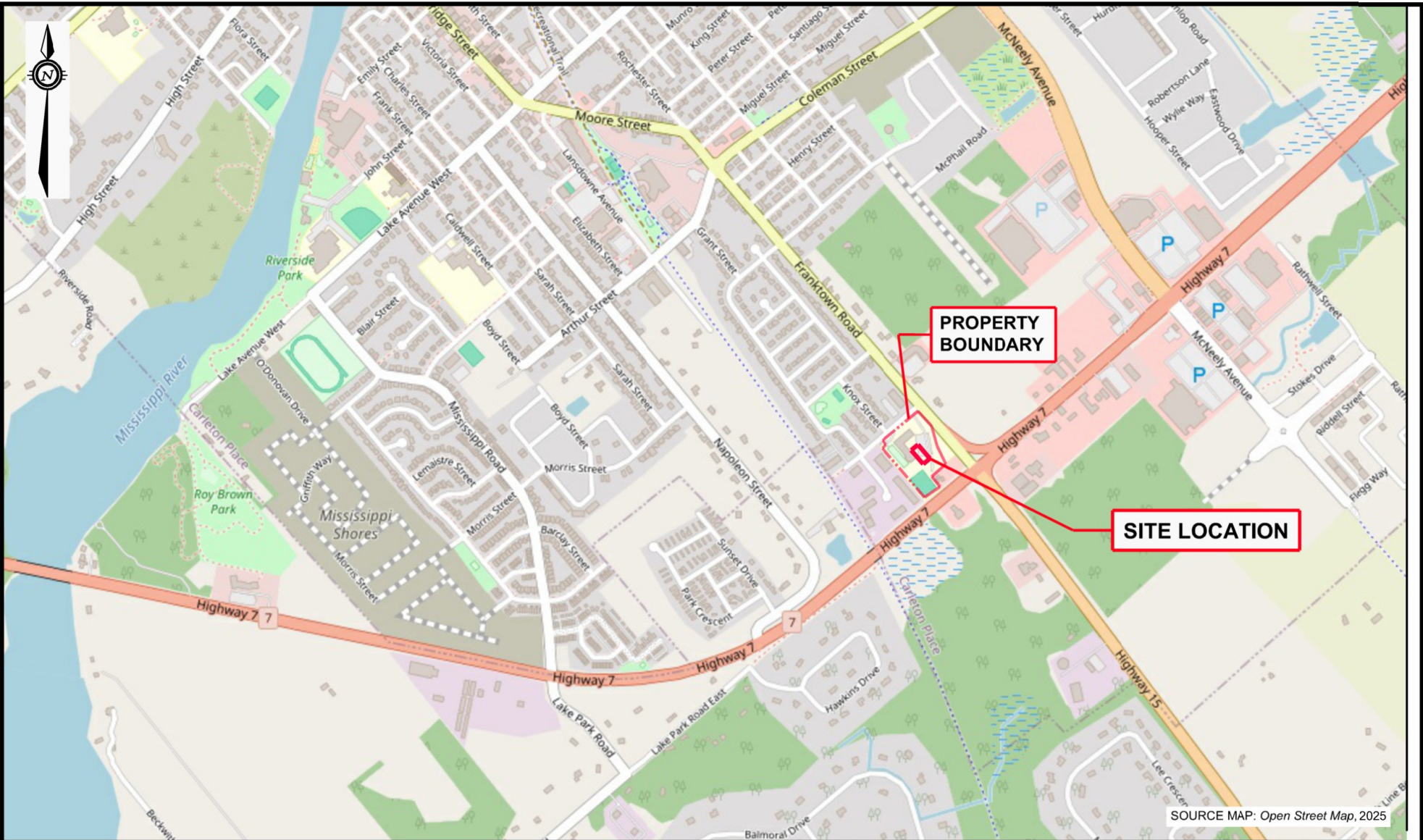


Ismail Taki, M. Eng., P.Eng.
Senior Manager, Eastern Region
Earth & Environment

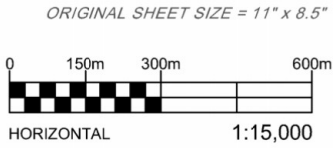
Project Name: Geotechnical Investigation - Proposed Seven Classroom Addition
J. L. Couroux Catholic Elementary School, 10 Findlay Avenue, Carleton Place, Ontario. *Project 2025JLC117*
OTT-25006852-A0
July 20, 2026

Figures

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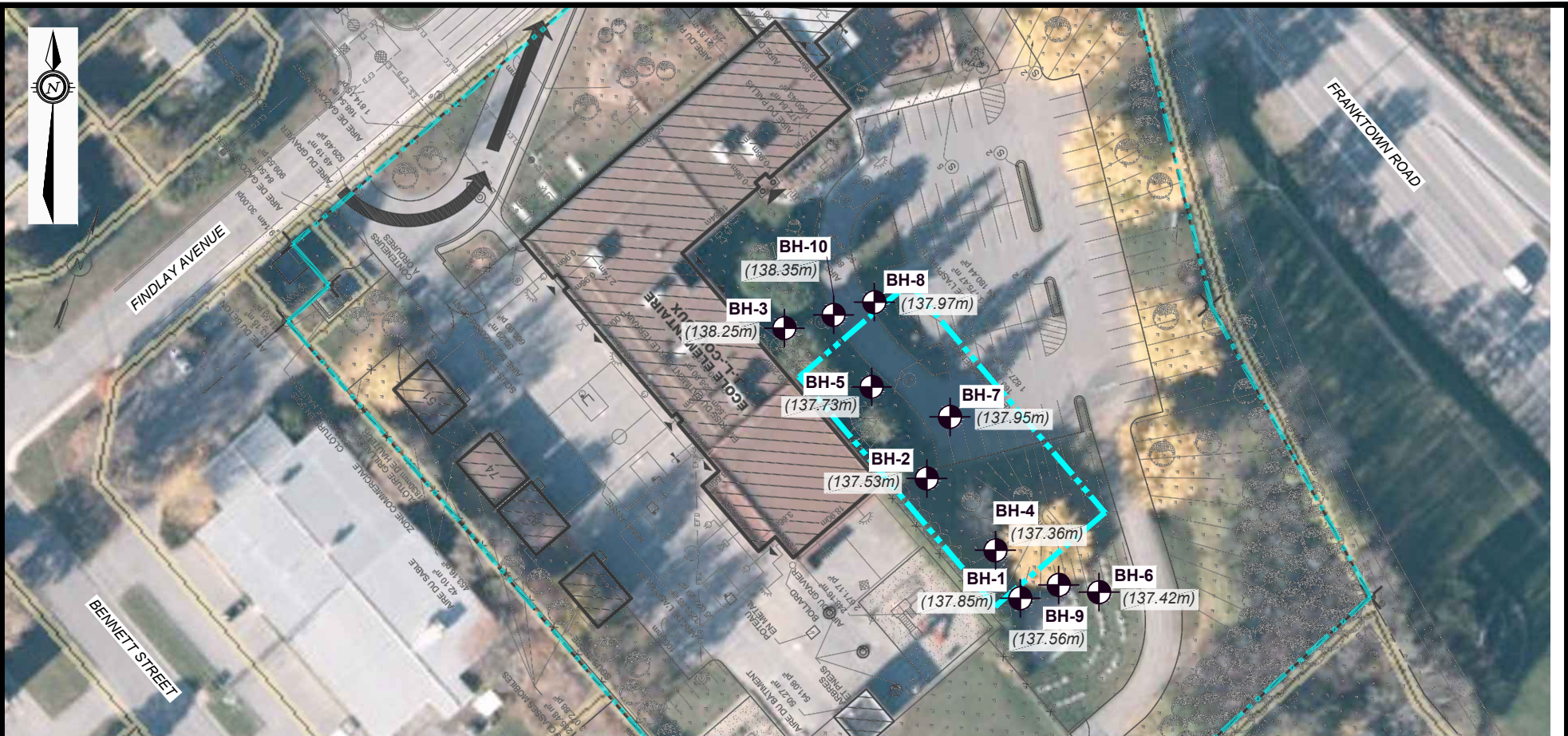
SOURCE MAP: Open Street Map, 2025



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DATE JUNE 2025		CLIENT: CONSEIL DES ÉCOLES CATHOLIQUES DU CENTRE-EST (CECCE)	project no. OTT-25006852-A0
DESIGN MZ	CHECKED IT	PROJECT: J. L. COUROUX SCHOOL ADDITION - GEOTECHNICAL INVESTIGATION	scale 1:15,000
DRAWN BY AS		ADDRESS: 10 FINDLAY AVENUE, CARLETON PLACE, ONTARIO SITE LOCATION PLAN	FIG 1

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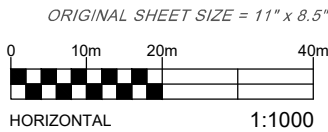
AERIAL PHOTOGRAPH SOURCE: AgMaps,
(www.liaapplications.lrc.gov.on.ca/AgMaps)

GENERAL NOTES:

1. THE BOUNDARIES, SOIL AND ROCK TYPES HAVE BEEN ESTABLISHED ONLY AT BOREHOLE LOCATIONS. BETWEEN BOREHOLES THEY ARE ASSUMED AND MAY BE SUBJECT TO CONSIDERABLE ERROR.
2. SOIL AND ROCK SAMPLES WILL BE RETAINED IN STORAGE FOR THREE MONTHS AND THEN DESTROYED UNLESS THE CLIENT ADVISES THAT AN EXTENDED TIME PERIOD IS REQUIRED.
3. TOPSOIL AND ASPHALT QUANTITIES SHOULD NOT BE ESTABLISHED FROM THE INFORMATION PROVIDED AT THE BOREHOLE LOCATIONS.
4. BOREHOLE ELEVATIONS SHOULD NOT BE USED TO DESIGN BUILDING(S) OR FLOOR SLABS OR PARKING LOT(S) GRADES.
5. THIS DRAWING FORMS PART OF THE REPORT PROJECT NUMBER AS REFERENCED AND SHOULD BE USED ONLY IN CONJUNCTION WITH THIS REPORT.
6. BASE PLAN INFORMATION OBTAINED FROM CECCE, FILE No.: JLC_371_I01, SHEET No.: I-1/1, DATED: 2018.08.29.

LEGEND

- APPROX. PROJECT BOUNDARY
- BH-1 BOREHOLE NO. AND LOCATION (2025)
(137.85m) GROUND SURFACE ELEVATION (m)

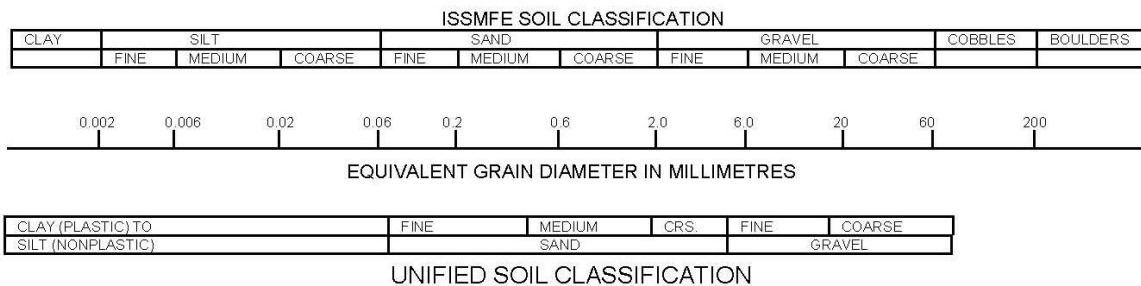


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DATE JUNE 2025		CLIENT: CONSEIL DES ÉCOLES CATHOLIQUES DU CENTRE-EST (CECCE)	project no. OTT-25006852-A0
DESIGN MZ	CHECKED IT	PROJECT: J. L. COUROY SCHOOL ADDITION - GEOTECHNICAL INVESTIGATION	scale 1:1,000
DRAWN BY AS		ADDRESS: 10 FINDLAY AVENUE, CARLETON PLACE, ONTARIO BOREHOLE LOCATION PLAN	FIG 2

Notes On Sample Descriptions

- All sample descriptions included in this report follow the Canadian Foundations Engineering Manual soil classification system. This system follows the standard proposed by the International Society for Soil Mechanics and Foundation Engineering. Laboratory grain size analyses provided by **exp** Services Inc. also follow the same system. Different classification systems may be used by others; one such system is the Unified Soil Classification. Please note that, with the exception of those samples where a grain size analysis has been made, all samples are classified visually. Visual classification is not sufficiently accurate to provide exact grain sizing or precise differentiation between size classification systems.



- Fill:** Where fill is designated on the borehole log it is defined as indicated by the sample recovered during the boring process. The reader is cautioned that fills are heterogeneous in nature and variable in density or degree of compaction. The borehole description may therefore not be applicable as a general description of site fill materials. All fills should be expected to contain obstruction such as wood, large concrete pieces or subsurface basements, floors, tanks, etc., none of these may have been encountered in the boreholes. Since boreholes cannot accurately define the contents of the fill, test pits are recommended to provide supplementary information. Despite the use of test pits, the heterogeneous nature of fill will leave some ambiguity as to the exact composition of the fill. Most fills contain pockets, seams, or layers of organically contaminated soil. This organic material can result in the generation of methane gas and/or significant ongoing and future settlements. Fill at this site may have been monitored for the presence of methane gas and, if so, the results are given on the borehole logs. The monitoring process does not indicate the volume of gas that can be potentially generated nor does it pinpoint the source of the gas. These readings are to advise of the presence of gas only, and a detailed study is recommended for sites where any explosive gas/methane is detected. Some fill material may be contaminated by toxic/hazardous waste that renders it unacceptable for deposition in any but designated land fill sites; unless specifically stated the fill on this site has not been tested for contaminants that may be considered toxic or hazardous. This testing and a potential hazard study can be undertaken if requested. In most residential/commercial areas undergoing reconstruction, buried oil tanks are common and are generally not detected in a conventional geotechnical site investigation.
- Till:** The term till on the borehole logs indicates that the material originates from a geological process associated with glaciation. Because of this geological process the till must be considered heterogeneous in composition and as such may contain pockets and/or seams of material such as sand, gravel, silt or clay. Till often contains cobbles (60 to 200 mm) or boulders (over 200 mm). Contractors may therefore encounter cobbles and boulders during excavation, even if they are not indicated by the borings. It should be appreciated that normal sampling equipment cannot differentiate the size or type of any obstruction. Because of the horizontal and vertical variability of till, the sample description may be applicable to a very limited zone; caution is therefore essential when dealing with sensitive excavations or dewatering programs in till materials.

Log of Borehole BH25-01



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 3

Page. 1 of 1

Date Drilled: June 6, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☒

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☒

Shear Strength by Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at % Strain at Failure ☐

Shear Strength by Penetrometer Test ☒

GWL	SYMBOL	SOIL DESCRIPTION	Geodetic Elevation m	Depth m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			SAMPLES	Natural Unit Wt. kN/m³	
					20	40	60	80	250	500	750			
					Shear Strength kPa				Natural Moisture Content % Atterberg Limits (% Dry Weight)					
			137.85	0	50	100	150	200		20	40	60		
		TOPSOIL ~ 125 mm thick	137.7											
		FILL Sand and gravel with silty clay pockets, rootlets, rock fragments, brown, damp to moist, no odours, no stains, (loose to dense)		7										SS1
			137.0											
		HIGHLY WEATHERED BEDROCK	136.9											SS2
		Auger Refusal at 1.0 m Depth		1										
		Note: 1) SS1 sample submitted for environmental laboratory analyses.												

NOTES:

- Borehole data requires interpretation by EXP before use by others
- Borehole backfilled upon completion of drilling.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %

Log of Borehole BH25-03



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 5

Page. 1 of 1

Date Drilled: June 5, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☒

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☐

Shear Strength by
Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at
% Strain at Failure ☐

Shear Strength by
Penetrometer Test ☒

GWL	SYMBOL	SOIL DESCRIPTION	Geodetic Elevation m	Depth m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			SAMPLES	Natural Unit Wt. kN/m³	
					20	40	60	80	250	500	750			
					Shear Strength kPa				Natural Moisture Content % Atterberg Limits (% Dry Weight)					
					50	100	150	200	20	40	60			
		TOPSOIL ~ 100 mm thick	138.25	0										
		FILL Silty sand with gravel and silty clay pockets, contains rock fragments, inclusions of topsoil, rootlets, brown, moist, no odours, no stains, (loose)	138.2	8 ○										SS1
		Augers grinding below 0.6 m depth on possible cobbles, boulders, or weathered bedrock		7, then 50 for 127 mm ○										SS2
		Auger Refusal at 1.1 m Depth	137.2	1										

NOTES:

- Borehole data requires interpretation by EXP before use by others
- Borehole backfilled upon completion of drilling.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %

Log of Borehole BH25-04



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 6

Page. 1 of 1

Date Drilled: June 5, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☒

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☒

Shear Strength by
Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at
% Strain at Failure ☐

Shear Strength by
Penetrometer Test ☒

G W L	S Y M B O L	SOIL DESCRIPTION	Geodetic Elevation m	D e p t h m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			S A M P L E S	Natural Unit Wt. kN/m³
					20	40	60	80	250	500	750		
					Shear Strength kPa				Natural Moisture Content % Atterberg Limits (% Dry Weight)				
					50	100	150	200	20	40	60		
		TOPSOIL ~125 mm thick	137.36										
		FILL Silty sand and gravel with silty clay pockets, topsoil inclusions, rootlets, rock fragments, brown, moist, no odours, no stains, (compact)	137.2										
		Auger Refusal at 0.6 m Depth	136.8										
		Note: 1) SS1 sample submitted for environmental laboratory analyses.											

NOTES:

- Borehole data requires interpretation by EXP before use by others
- Borehole backfilled upon completion of drilling.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %

Log of Borehole BH25-05



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 7

Page. 1 of 1

Date Drilled: June 5, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☒

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☒

Shear Strength by Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at % Strain at Failure ☐

Shear Strength by Penetrometer Test ☒

GWL	SYMBOL	SOIL DESCRIPTION	Geodetic Elevation m	Depth m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			SAMPLES	Natural Unit Wt. kN/m³
					20	40	60	80	250	500	750		
					Shear Strength kPa				Natural Moisture Content % Atterberg Limits (% Dry Weight)				
					50	100	150	200	20	40	60		
		TOPSOIL ~ 200 mm thick	137.73	0									
		FILL Silty sand and gravel with silty clay pockets, inclusions of topsoil, rootlets, rock fragments, dark brown to brown, damp to moist, no odours, no stains, (very loose to dense)	137.5	3									SS1
				6, then 50 for 50mm									SS2
		Auger Refusal at 1.0 m Depth	136.7	1									
		Note: 1) SS2 sample submitted for environmental laboratory analyses.											

NOTES:

- Borehole data requires interpretation by EXP before use by others
- Borehole backfilled upon completion of drilling.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

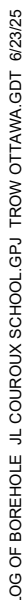
Date	Water Level (m)	Hole Open To (m)

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %

Page. 1 of 1

Shear Strength by Penetrometer Test



Run No.	Depth (m)	% Rec.	RQD %
1	2.9 - 3.1	100	100
2	3.1 - 4.6	93	75

Log of Borehole BH25-07



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 9

Page. 1 of 1

Date Drilled: June 5, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☒

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☒

Shear Strength by Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at % Strain at Failure ☐

Shear Strength by Penetrometer Test ☒

GWL	SYMBOL	SOIL DESCRIPTION	Geodetic Elevation m	Depth m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			SAMPLES	Natural Unit Wt. kN/m³	
					20	40	60	80	250	500	750			
					Shear Strength				Natural Moisture Content %					
					kPa				Atterberg Limits (% Dry Weight)					
					50	100	150	200		20	40	60		
		ASPHALT ~ 40 mm thick	137.95	0	16					X				SS1
		GRANULAR FILL ~110 mm thick Crusher run, grey, damp, (compact)	137.9 137.8											
		FILL Silty sand and gravel, inclusions of rock fragments, rootlets, and topsoil, brown, damp to moist, no odours, no stains, (compact)		1	16					X				SS2
		Augers grinding below 0.6 m depth on possible cobbles, boulders, or weathered bedrock	136.5											
		Auger Refusal at 1.5 m Depth												
		Note: 1) SS2 sample submitted for environmental laboratory analyses.												

NOTES:

- Borehole data requires interpretation by EXP before use by others
- Borehole backfilled upon completion of drilling.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %

Log of Borehole BH25-08



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 10

Page. 1 of 1

Date Drilled: June 5, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☐

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☐

Shear Strength by Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at % Strain at Failure ☐

Shear Strength by Penetrometer Test ☐

GWL	SYMBOL	SOIL DESCRIPTION	Geodetic Elevation m	Depth m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			SAMPLES	Natural Unit Wt. kN/m³
					20	40	60	80	250	500	750		
					Shear Strength kPa				Natural Moisture Content % Atterberg Limits (% Dry Weight)				
					50	100	150	200	20	40	60		
		TOPSOIL ~85 mm thick	137.97	0									
		FILL Silty sand with gravel, inclusions of rootlets, rock fragments and topsoil, moist, dark brown, no odours, no stains, (loose)	137.9	0	2, then 50 for 125mm								
		DOLOSTONE BEDROCK Sandy, with sandstone partings, grey and reddish brown	137.6	0									SS1
				1									
				2									
				3									
			134.97										
			134.9										
		Borehole Terminated at 3.1 m Depth											
									</				

NOTES:

- Borehole data requires interpretation by EXP before use by others
- A 19 mm diameter standpipe installed as shown.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)
June 16, 2025	3.0	

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %
1	0.4 - 1.6	89	51
2	1.6 - 3.1	85	68

LOG OF BOREHOLE - JL COURNOUX SCHOOL GPJ TROW OTTAWA.GDT 6/23/25

Log of Borehole BH25-09



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 11

Page. 1 of 1

Date Drilled: June 6, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☐

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☐

Shear Strength by
Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at
% Strain at Failure ☐

Shear Strength by
Penetrometer Test ☐

G W L	S Y M B O L	SOIL DESCRIPTION	Geodetic Elevation m	D e p t h m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			S A M P L E S	Natural Unit Wt. kN/m³
					20	40	60	80	250	500	750		
					Shear Strength kPa				Natural Moisture Content % Atterberg Limits (% Dry Weight)				
					50	100	150	200	20	40	60		
		TOPSOIL ~ 175 mm thick	137.56	0									
		FILL Silty sand and gravel, with rock fragments and topsoil inclusions, moist, brown, no odours, no stains, (loose)	137.4	4						X			SS1
		Auger Refusal at 0.8 m Depth	136.8										

NOTES:

- Borehole data requires interpretation by EXP before use by others
- Borehole backfilled upon completion of drilling.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %

Log of Borehole BH25-10



Project No: OTT-25006852-A0

Project: Proposed Classroom Addition - École Élémentaire Catholique J.-L.-Couroux

Location: 10 Findlay Avenue, Carleton Place, Ontario

Figure No. 12

Page. 1 of 1

Date Drilled: June 5, 2025

Drill Type: CME 55 LC- Track Mounted Drill Rig

Datum: Geodetic Elevation

Logged by: SA Checked by: MZ

Split Spoon Sample ☒

Auger Sample ☒

SPT (N) Value ☐

Dynamic Cone Test ☐

Shelby Tube ☒

Shear Strength by Vane Test ☐

Combustible Vapour Reading ☐

Natural Moisture Content ☒

Atterberg Limits ☐

Undrained Triaxial at % Strain at Failure ☐

Shear Strength by Penetrometer Test ☒

GWL	SYMBOL	SOIL DESCRIPTION	Geodetic Elevation m	Depth m	Standard Penetration Test N Value				Combustible Vapour Reading (ppm)			SAMPLES	Natural Unit Wt. kN/m³	
					20	40	60	80	250	500	750			
					Shear Strength kPa				Natural Moisture Content % Atterberg Limits (% Dry Weight)					
					50	100	150	200	20	40	60			
		TOPSOIL ~100 mm thick	138.35	0										
		FILL Silty sand and gravel, with rock fragments and topsoil inclusions, brown, moist, no odours, no stains, (compact)	138.3		13									SS1
		Augers grinding below 0.6 m on possible cobbles, boulders, or weathered bedrock												
		HIGHLY WEATHERED BEDROCK	137.4	1										SS2
		Auger Refusal at 1.2 m Depth	137.2											

NOTES:

- Borehole data requires interpretation by EXP before use by others
- Borehole backfilled upon completion of drilling.
- Field work supervised by an EXP representative.
- See Notes on Sample Descriptions
- Log to be read with EXP Report OTT-25006852-A0

WATER LEVEL RECORDS

Date	Water Level (m)	Hole Open To (m)

CORE DRILLING RECORD

Run No.	Depth (m)	% Rec.	RQD %

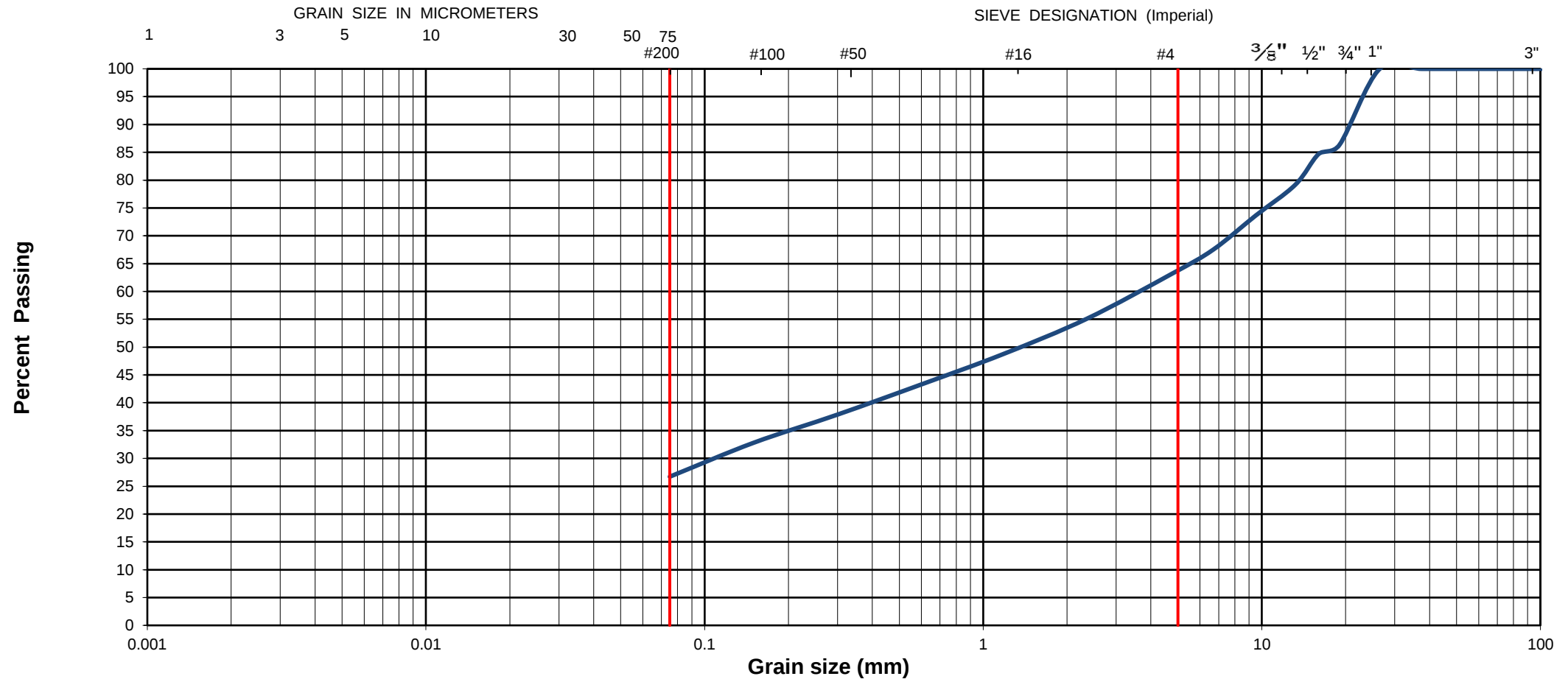


Grain-Size Distribution Curve
Method of Test For Sieve Analysis of Aggregate
ASTM C-136

EXP Services Inc.
100-2650 Queensview Drive
Ottawa, ON K2B 8H6

Unified Soil Classification System

CLAY AND SILT	SAND			GRAVEL	
	Fine	Medium	Coarse	Fine	Coarse



EXP Project No.:	OTT-25006852-A0	Project Name :	Proposed Classroom Addition - J.L. Couroux Catholic Elementary School			
Client :	CECCE	Project Location :	10 Findlay Avenue, Carleton Place, Ontario			
Date Sampled :	June 6, 2025	Borehole No:	BH6	Sample:	SS2	Depth (m) : 0.8 - 1.4
Sample Composition :	Gravel (%)	37	Sand (%)	36	Silt & Clay (%)	27
Sample Description :	FILL: Silty Sand and Gravel (SM / GM)					Figure : 13

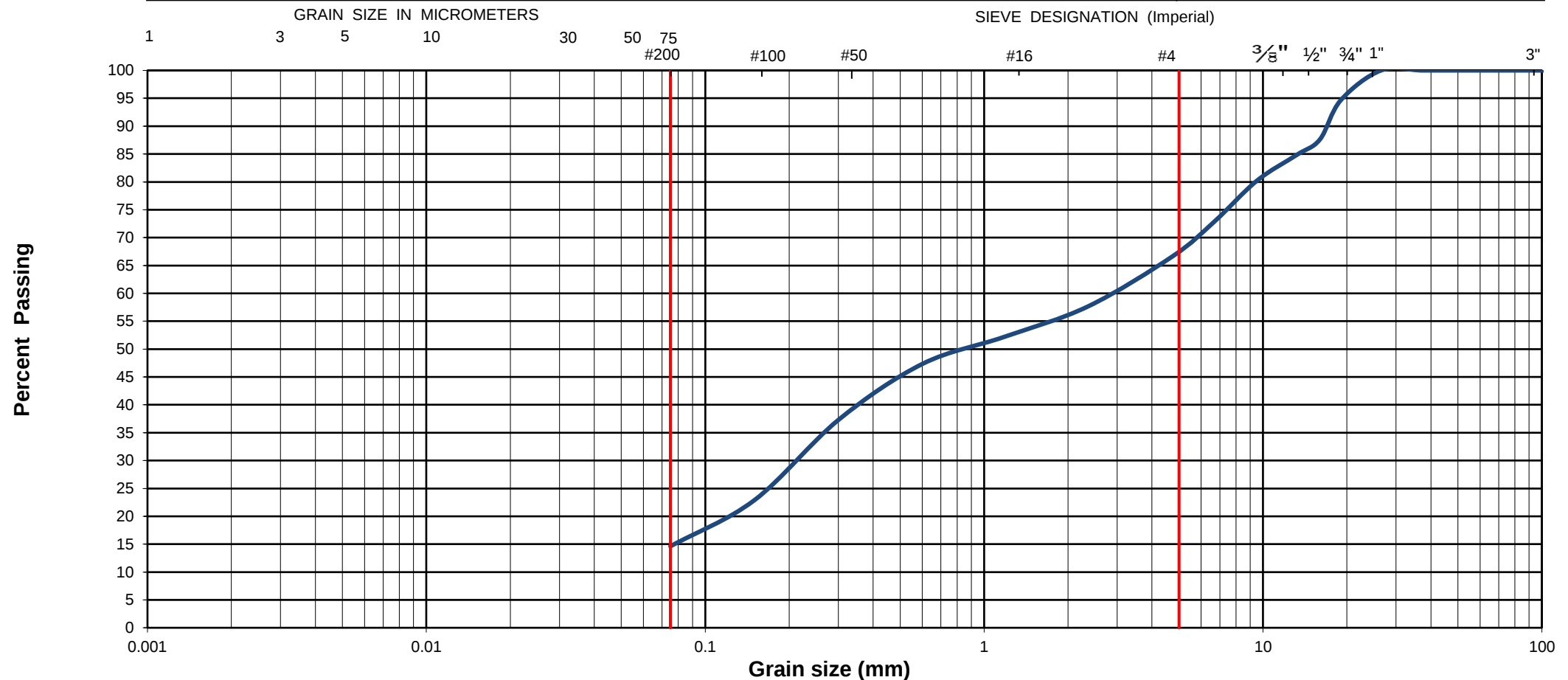


Grain-Size Distribution Curve
Method of Test For Sieve Analysis of Aggregate
ASTM C-136

EXP Services Inc.
100-2650 Queensview Drive
Ottawa, ON K2B 8H6

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CLAY AND SILT	SAND			GRAVEL	
	Fine	Medium	Coarse	Fine	Coarse

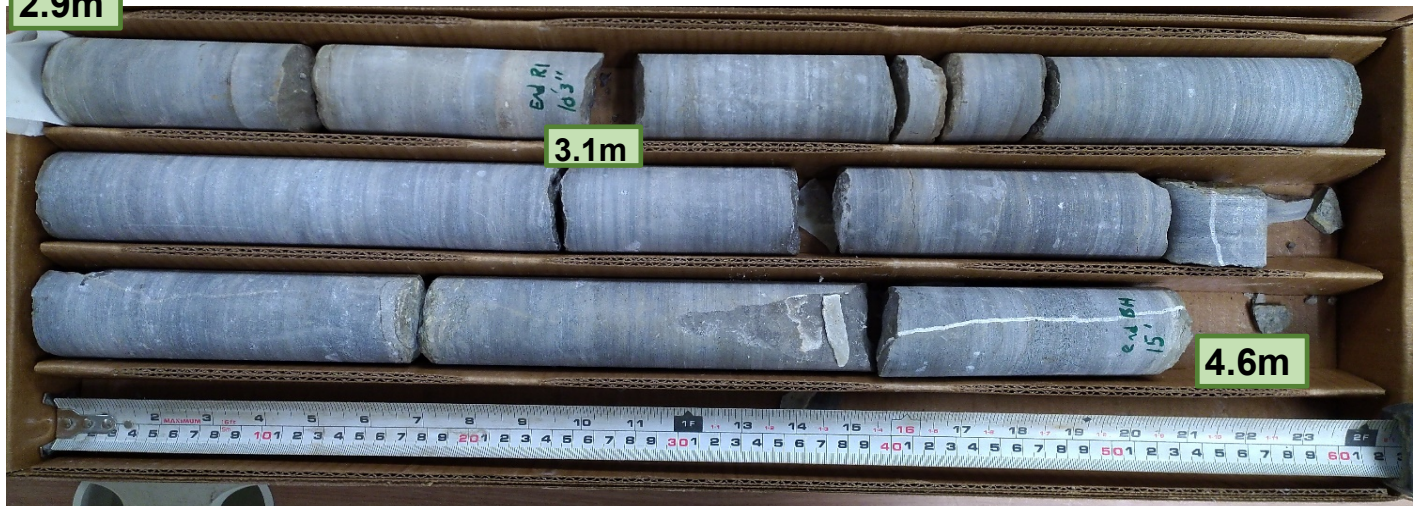


EXP Project No.:	OTT-25006852-A0	Project Name :	Proposed Classroom Addition - J.L. Couroux Catholic Elementary School			
Client :	CECCE	Project Location :	10 Findlay Avenue, Carleton Place, Ontario			
Date Sampled :	June 6, 2025	Borehole No:	BH10	Sample:	SS1	Depth (m) : 0 - 0.6
Sample Composition :	Gravel (%)	33	Sand (%)	52	Silt & Clay (%)	15
Sample Description :	FILL: Silty Sand (SM), Gravelly					Figure : 14

Appendix A – Bedrock Core Photographs

DRY BEDROCK CORES

2.9m



WET BEDROCK CORES

2.9m



exp Services Inc.

t: +1.613.688.1899 | f: +1.613.225.7337
2650 Queensview Drive, Suite 100
Ottawa, ON K2B 8H6
Canada

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borehole no. BH25-6	core runs Run 1: 2.9m - 3.1m Run 2: 3.1m - 4.6m end of borehole	PROJECT	Proposed Classroom Addition - École Élémentaire Catholique J.L. Couroux 10 Findlay Avenue, Carleton Place, Ontario	project no. OTT-25006852-A0
date cored Jun 06, 2025			ROCK CORE PHOTOGRAPHS	FIG A1

0.4m

DRY BEDROCK CORES

1.6m

3.1m

0.4m

WET BEDROCK CORES

1.6m

3.1m



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borehole no. BH25-8	core runs Run 1: 0.4m - 1.6m Run 2: 1.6m - 3.1m end of borehole	PROJECT	Proposed Classroom Addition - École Élémentaire Catholique J.L. Couroux 10 Findlay Avenue, Carleton Place, Ontario	project no. OTT-25006852-A0
date cored Jun 05, 2025			ROCK CORE PHOTOGRAPHS	FIG A2

Project Name: Geotechnical Investigation - Proposed Seven Classroom Addition
J. L. Couroux Catholic Elementary School, 10 Findlay Avenue, Carleton Place, Ontario. Project 2025JLC117
OTT-25006852-A0
July 20, 2026

Appendix B – Laboratory Certificate of Analysis

CLIENT NAME: EXP SERVICES INC
2650 QUEENSVIEW DRIVE, UNIT 100
OTTAWA, ON K2B8H6
(613) 688-1899

ATTENTION TO: Matthew Zammit

PROJECT: OTT-25006852-AO

AGAT WORK ORDER: 25Z310935

SOIL ANALYSIS REVIEWED BY: Sukhwinder Randhawa, Inorganic Team Lead

DATE REPORTED: Jun 26, 2025

PAGES (INCLUDING COVER): 5

VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

*Notes

Disclaimer:

- All work conducted herein has been done using accepted standard protocols, and generally accepted practices and methods. AGAT test methods may incorporate modifications from the specified reference methods to improve performance.
- All samples will be disposed of within 30 days after receipt unless a Long Term Storage Agreement is signed and returned. Some specialty analysis may be exempt, please contact your Client Project Manager for details.
- AGAT's liability in connection with any delay, performance or non-performance of these services is only to the Client and does not extend to any other third party. Unless expressly agreed otherwise in writing, AGAT's liability is limited to the actual cost of the specific analysis or analyses included in the services.
- This Certificate shall not be reproduced except in full, without the written approval of the laboratory.
- The test results reported herewith relate only to the samples as received by the laboratory.
- Application of guidelines is provided "as is" without warranty of any kind, either expressed or implied, including, but not limited to, warranties of merchantability, fitness for a particular purpose, or non-infringement. AGAT assumes no responsibility for any errors or omissions in the guidelines contained in this document.
- All reportable information is available on request from AGAT Laboratories, in accordance with ISO/IEC 17025:2017, ISO/IEC 17025:2005 (Quebec), DR-12-PALA and/or NELAP Standards.
- This document is signed by an authorized signatory who meets the requirements of the MELCCFP, CALA, CCN and NELAP.
- For environmental samples in the Province of Quebec: The analysis is performed on and results apply to samples as received. A temperature above 6°C upon receipt, as indicated in the Sample Reception Notification (SRN), could indicate the integrity of the samples has been compromised if the delay between sampling and submission to the laboratory could not be minimized.



AGAT Laboratories

Certificate of Analysis

AGAT WORK ORDER: 25Z310935

PROJECT: OTT-25006852-AO

5835 COOPERS AVENUE
MISSISSAUGA, ONTARIO
CANADA L4Z 1Y2
TEL (905)712-5100
FAX (905)712-5122
<http://www.agatlabs.com>

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE: J.L. Couroux School

ATTENTION TO: Matthew Zammit

SAMPLED BY: EXP

(Soil) Inorganic Chemistry

DATE RECEIVED: 2025-06-17

DATE REPORTED: 2025-06-26

		SAMPLE DESCRIPTION:		BH8 Run
		SAMPLE TYPE:		1'19"-2'1"
		DATE SAMPLED:		Other
				2025-06-06
Parameter	Unit	G / S	RDL	6824494
Chloride (2:1)	µg/g	2	40	
Sulphate (2:1)	µg/g	2	13	
pH (2:1)	pH Units	NA	9.71	
Resistivity (2:1) (Calculated)	ohm.cm	1	6210	

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard

6824494 pH, Chloride and Sulphate were determined on the extract obtained from the 2:1 leaching procedure (2 parts DI water: 1 part soil). Resistivity is a calculated parameter.

Analysis performed at AGAT Toronto (unless marked by *)

Certified By:



Quality Assurance

CLIENT NAME: EXP SERVICES INC
PROJECT: OTT-25006852-AO
SAMPLING SITE: J.L Couroux School

AGAT WORK ORDER: 25Z310935
ATTENTION TO: Matthew Zammit
SAMPLED BY: EXP

Soil Analysis

RPT Date: Jun 26, 2025			DUPLICATE			Method Blank	REFERENCE MATERIAL			METHOD BLANK SPIKE			MATRIX SPIKE		
PARAMETER	Batch	Sample Id	Dup #1	Dup #2	RPD		Measured Value	Acceptable Limits		Recovery	Acceptable Limits		Recovery	Acceptable Limits	
								Lower	Upper		Lower	Upper		Lower	Upper
(Soil) Inorganic Chemistry															
Chloride (2:1)	6843227		424	410	3.4%	< 2	97%	70%	130%	97%	80%	120%	NA	70%	130%
Sulphate (2:1)	6843227		78	76	2.6%	< 2	97%	70%	130%	93%	80%	120%	94%	70%	130%
pH (2:1)	6835270		8.18	8.41	2.8%	NA	99%	80%	120%						

Comments: NA signifies Not Applicable.

pH duplicates QA acceptance criteria was met relative as stated in Table 5-15 of Analytical Protocol document.

Duplicate NA: results are under 5X the RDL and will not be calculated.

Matrix spike NA: Spike level < native concentration. Matrix spike acceptance limits do not apply and are not calculated.

Certified By: _____





Method Summary

CLIENT NAME: EXP SERVICES INC

PROJECT: OTT-25006852-AO

SAMPLING SITE: J.L Couroux School

AGAT WORK ORDER: 25Z310935

ATTENTION TO: Matthew Zammit

SAMPLED BY: EXP

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Soil Analysis			
Chloride (2:1)	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
Sulphate (2:1)	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
pH (2:1)	INOR 93-6031	modified from EPA 9045D and MCKEAGUE 3.11	PH METER
Resistivity (2:1) (Calculated)	INOR-93-6036	McKeague 4.12, SM 2510 B, SSA #5 Part 3	CALCULATION



Chain of Custody Record

If this is a Drinking Water sample, please use Drinking Water Chain of Custody Form (potable water consumed by humans)

Report Information:

Company: EXP Services Inc
Contact: Matthew Zammit
Address: 2650 Queensview Drive, Suite 100
Ottawa, Ontario, K2B 8H6
Phone: 613-688-1899 Fax: _____
Reports to be sent to:
1. Email: matthew.zammit@exp.com
2. Email: ryan.digiuseppe@exp.com

Project Information:

Project: OTT-25006852-A0
Site Location: J.L. Couroux School
Sampled By: EXP
AGAT Quote #: _____ PO: _____
Please note: If quotation number is not provided, client will be billed full price for analysis.

Invoice Information:

Bill To Same: Yes ☒ No ☐

Company: _____
Contact: _____
Address: _____
Email: _____

Regulatory Requirements:

(Please check all applicable boxes)

☐ Regulation 153/04	☐ Regulation 406	☐ Sewer Use
☐ Ind/Com	☐ Ind/Com	☐ Sanitary ☐ Storm
☐ Res/Park	☐ Res/Park	☐ Prov. Water Quality Objectives (PWQO)
☐ Agriculture	☐ Agriculture	☐ Other
Soil Texture (Check One)	☐ Regulation 558	Region _____
☐ Coarse	☐ CCME	Indicate One
☐ Fine		

Is this submission for a Record of Site Condition (RSC)?

☐ Yes ☐ No

Report Guideline on Certificate of Analysis

☐ Yes ☐ No

Legal Sample ☐

Sample Matrix Legend

GW Ground Water SD Sediment
O Oil SW Surface Water
P Paint R Rock/Shale
S Soil

Laboratory Use Only

Work Order #: 252310935
Cooler Quantity: N/A
Arrival Temperatures: 31.2 | 30.1 | 31.4
Depot Temperatures: 8.2 | 8.1 | 7.3
Custody Seal Intact: ☐ Yes ☐ No ☒ N/A
Notes: B I T

Turnaround Time (TAT) Required:

Regular TAT ☒ 5 to 7 Business Days

Rush TAT (Rush Surcharges Apply)

☐ 3 Business Days ☐ 2 Business Days ☐ Next Business Day

OR Date Required (Rush Surcharges May Apply):

Please provide prior notification for rush TAT
*TAT is exclusive of weekends and statutory holidays

For 'Same Day' analysis, please contact your AGAT CSR

Sample Identification		Date Sampled	Time Sampled	# of Containers	Sample Matrix	Comments/ Special Instructions	Y / N	Metals	Metals	BTEX, F	VOC	PAHs	PCBs: A	Regulat	pH, Met	EC, SAR	Regulat	mSPLP:	Landfill	TCLP: ☐	Corrosi	pH	Sulp	Chlo	Resi		Potentia
1.	BH8 Run1 1'9" - 2'1"	June 6	AM PM	1																		☒	☒	☒	☒		
2.			AM PM																								
3.			AM PM																								
4.			AM PM																								
5.			AM PM																								
6.			AM PM																								
7.			AM PM																								
8.			AM PM																								
9.			AM PM																								
10.			AM PM																								
11.			AM PM																								

Samples Relinquished By (Print Name and Sign):	Date:	Time:	Samples Received By (Print Name and Sign):	Date:	Time:
Linda Berthel	2005-06-07	15:00	Tiffan	June 18	9 AM
Samples Relinquished By (Print Name and Sign):	Date:	Time:	Samples Received By (Print Name and Sign):	Date:	Time:
Samples Relinquished By (Print Name and Sign):	Date:	Time:	Samples Received By (Print Name and Sign):	Date:	Time:

Page _____ of _____

N/A

Appendix C – Legal Notification

Legal Notification

This report was prepared by EXP Services for the account of Conseil des Écoles Catholiques du Centre-Est.

Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. EXP Services Inc. accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this project.

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